

Stormwater Management Report

Owner:

United Properties
250 Nicollet Mall, #500
Minneapolis, MN 55401

Project:

Amira Villas Minnetonka
901 Carlson Parkway
Minnetonka, MN 55305

Engineer's Certification:

All plans and supporting Documentation contained in this report have been reviewed by me and it is hereby certified that to the best of my knowledge the plans comply with the requirements of the ordinance.

I hereby certify that this report was prepared by me or under my direct supervision and that I am a duly Registered Professional Engineer under the laws of the State of Minnesota.



David J. Knaeble P.E.

Registration Number: 48776

Date:

6/7/24

Table of Contents:

1.0	Cover Sheet, Engineer's Certification
2.0	Summary Analysis / Narrative
2.1	Introduction
2.2	Existing Site Conditions
2.3	Proposed Site Conditions
2.4	Stormwater Requirements – City
2.5	Stormwater Requirements – Watershed District
2.6	Stormwater Requirements – MPCA / NPDES
3.0	Stormwater Calculations
3.1	Proposed Stormwater Management Strategy & Facilities Description
3.2	Pretreatment Practices Proposed
3.3	Rate Control
3.4	Volume Control
3.5	Water Quality
4.0	Conclusions

Figures:

Figure 1 – Drainage Calculations Summary Table
Figure 2 – Momentum Preserver Calculation Results
Figure 3 – P8 Calculator Results
Figure 4 – Existing Conditions Drainage Area Map
Figure 5 – Proposed Conditions Drainage Area Map

Appendices:

- HydroCAD
 - Existing Conditions HydroCAD Modeling
 - Proposed Conditions HydroCAD Modeling
- Storm Sewer Sizing [TO BE PROVIDED AT A FUTURE DATE]
 - Storm Sewer Sizing Map
 - Storm Sewer Sizing Spreadsheet
- Geotechnical Report (On File at the Office of the Engineer)

2.0 Summary Analysis / Narrative:

2.1 Introduction:

This stormwater management report accompanies the Civil Engineering Plans prepared by Civil Site Group for the subject project dated 6/7/24. This report includes a summary of the existing and proposed site conditions, the stormwater requirements of relevant regulatory agencies, and proposed design calculations and data to meet the requirements.

2.2 Existing Site Conditions :

Site Description:

The existing site is a vacant lot adjacent to a recently constructed apartment building. Below is the existing surface area tabulation.

Existing Conditions						
Drainage Area	Impervious Area		Pervious Area		Total Area	
	Area [SF]	CN Value	Area [SF]	CN Value	Area [SF]	CN Value
EX1	9729	98	51485	80	61214	83
EX2	687	98	39156	80	39843	80

Existing Soils:

A geotechnical evaluation report was completed by Braun Intertec Corporation dated April 19, 2024. This report determined the soils on site are a mix of topsoil and fill soils consisting of clayey sands, fat clays and lean clays and sandy lean clays. For the purposes of this report, soils have been assumed to have a hydrologic soil group "D" designation and are not conducive to infiltration.

Groundwater:

Groundwater was only observed in one of the soil borings at an approximate elevation of 963. This was deemed to be a perched condition and the groundwater was expected to be below that elevation. The deepest boring reached an approximate elevation of 945.7.

2.3 Proposed Site Conditions:

Site Description:

The proposed site is a multi-building rental townhome community with drive lanes, landscaping, utility and stormwater improvements.

The proposed site surface coverage areas are shown in the table below:

Proposed Conditions						
Drainage Area	Impervious Area		Pervious Area		Total Area	
	Area [SF]	CN Value	Area [SF]	CN Value	Area [SF]	CN Value
PR1A	61334	98	12140	80	73474	95
PR1B	4181	98	10312	80	14493	85
PR2	2797	98	10293	80	13090	84

2.4 Stormwater Requirements City (Minnetonka):

Requirement threshold - Projects that disturb 50 cubic yards of earth or more, or disturb 5,000 square feet of surface area.

Rate Control – Limit peak runoff flow rates to existing conditions for 2-YR, 10-YR, and 100-YR frequency storms (nested 24-hour distribution).

Water Quality – If unique, site-specific constraints preclude infiltration, 60% TP and 90% TSS removal and no net increase in TP or TSS annually must be met.

Volume Control – Retain 1.1 inches of runoff on-site from applicable impervious surfaces.

2.5 Stormwater Requirements Watershed District – (Minnehaha Creek Watershed District):

Requirement threshold – New development sites larger than one acre and that will result in more than 20% impervious surface over the site.

Rate Control – No net increase in the peak runoff rate for the 1-, 10- and 100-year design storms where stormwater discharges across the downgradient site boundary, compared to the rate for the site in its existing condition.

Water Quality – No net increase in phosphorus loading from existing conditions.

Volume Control – Abstraction of the first inch of rainfall from the site’s impervious surface.

MCWD is not a reviewing authority for projects within the City of Minnetonka. The City of Minnetonka acts as the LGU and issues the permits.

2.6 Stormwater Requirements - Minnesota Pollution Control Agency – NPPDES permit (MPCA):

Requirement threshold - A permit is required for projects with a disturbed area over 1 acre in size, Stormwater management is required for a project adding 1-acre of more of NEW impervious surface (reconstructed impervious is not included).

Rate Control – No specific regulation, may not degrade downstream facilities.

Water Quality – Stormwater water quality treatment volume must be provided equal to 1.0” over all new impervious surfaces (includes all newly constructed impervious surfaces only, re-constructed impervious surfaces are not included).

Volume Control – Must consider volume reduction if feasible and not prohibited on site. The required infiltration volume is equal to the water quality volume described above.

3.0 Stormwater Calculations:

3.1 Proposed Stormwater Management Strategy & Facilities Description

This project is disturbing approximately 2.2 acres of land. The project will be constructing approximately 1.6 acres of new/reconstructed impervious surfaces. This land disturbance and creation of impervious surfaces will trigger stormwater management requirements from the City of Minnetonka and for the MPCA NPDES Permit.

This project is proposing to use underground detention basin along with pretreatment structures to meet the project requirements.

The underground CMP infiltration basin is designed to provide City of Minnetonka fire truck loading requirements of 83,000 lbs and 10,800 pounds per square foot outrigger point loading.

Below is a table from the manufacturer that indicates the fire truck loading can be provided by meeting minimum cover.

The proposed project will be utilizing a 84” pipe with a minimum cover of 36”.

Firetruck Loading

Please use the table below for general guidance.

HEL-COR® CSP Minimum Height of Cover Requirements for Firetruck Loading ¹					
Pipe Span, Inches	Corrugation Profile, Inches	Minimum Cover, Inches for Firetruck Outrigger Load (64 kips) ^{2,3}			
		16 GA	14 GA	12 GA	10 GA
		0.064	0.079	0.109	0.138
12 - 36	2 7/8 x 1/2	12	12	12	12
42 - 48	2 7/8 x 1/2	18	18	18	18
54 - 60	3 x 1 or 5 x 1	24	18		
72	3 x 1 or 5 x 1	30	24		
78 - 120	3 x 1 or 5 x 1		36	30	
126 - 144	3 x 1 or 5 x 1			42	36

1. Minimum cover may vary depending on local conditions. The contractor must provide additional cover required to avoid damage to the pipe. **Minimum cover is measured from the top of the pipe to the top of the maintained construction roadway surface.**
2. Table is based on a typical 85,000 lb GVW firetruck with an outrigger load of 64,000 lbs. The 64,000 lb outrigger force is applied over a surface area of about 850.6 in². The dimensions of the outrigger square pad are 25-7/8" x 32-7/8".
3. The outrigger load will be the heaviest load applied from the firetruck.

3.2 Pretreatment Practices Proposed

The project will utilize sumped storm structures along with Momentum Preservers to function as pretreatment for the detention basin. Baffle walls and drawdown weirs will also be utilized within the system itself at locations where inlets are directly over the top of the system. The goal of the pretreatment devices is to remove an adequate amount of TSS (total suspended solids) from runoff prior to stormwater entering the infiltration device. Pretreating significantly reduces the frequency and invasiveness of maintenance for the infiltration device.

Calculations provided by the manufacturer of the Momentum Preserver pretreatment devices indicate that this system can remove approximately 67.9% of the TSS prior to the stormwater reaching the system. This has been utilized in the water quality modeling section below.

Outputs:										
Annualized Pollutant Removal	Runoff Load (lbs.)	Bypassed Load (lbs.)	Load Removed, Excluding Scour (lbs.)	Removal Efficiency, Excluding Scour (%)	Scour Load (lbs.)	Load Removed, Including Scour (lbs.)	Removal Efficiency, Including Scour (%)	Pollutant Storage Depth (in)	Recommended Angle Between Pipes	Minimum Angle Between Pipes
	461		319	69.1%	6	313	67.9%	1.2	Future Version	
Device-Specific Design Assumptions → (See "Devices" tab for details)		Pipe Invert Elevations	Max. Pollutant Depth							

3.3 Rate Control

Limit peak runoff flow rates to existing conditions for 2-YR, 10-YR, and 100-YR frequency storms (nested 24-hour distribution)

Rate control is provided by live storage within the proposed underground detention basin. This information was derived using HydroCAD stormwater modeling software. The existing and proposed runoff rates are shown in the summary table below.

Stormwater Rate Summary

Drainage Area	Existing Rate (cfs)			
	1-YR [2.50"]	2-YR [2.88"]	10-YR [4.29"]	100-YR [7.42"]
EX1	2.76	3.50	6.43	13.28
EX2	1.54	2.01	3.91	8.39
TOTAL	4.30	5.51	10.34	21.67

Drainage Area	Proposed Conditions Rate (cfs)			
	1-YR [2.50"]	2-YR [2.88"]	10-YR [4.29"]	100-YR [7.42"]
1P (PR1A)	2.02	2.90	6.18	11.30
PR1B	0.74	0.92	1.61	3.23
PR2	0.62	0.78	1.41	2.87
TOTAL	3.38	4.60	9.20	17.40

Overall Stormwater Rate Summary

	Existing Conditions Rate (cfs)	Proposed Conditions Rate (cfs)
1-Year Event	4.30	3.38
2-Year Event	5.51	4.60
10-Year Event	10.34	9.20
100-Year Event	21.67	17.40

Stormwater Rate Summary - DA 1 - To Existing Storm Sewer South of Site

	Existing Conditions Rate (cfs)	Proposed Conditions Rate (cfs)
1-Year Event	2.76	2.76
2-Year Event	3.50	3.82
10-Year Event	6.43	7.79
100-Year Event	13.28	14.53

Stormwater Rate Summary - DA 2 - To Amira Apts North of Site

	Existing Conditions Rate (cfs)	Proposed Conditions Rate (cfs)
1-Year Event	1.54	0.62
2-Year Event	2.01	0.78
10-Year Event	3.91	1.41
100-Year Event	8.39	2.87

The existing project site discharges to two separate drainage subcatchments. Drainage Area DA1 discharges south towards the storm sewer system in Deer Creek Parkway. Drainage Area DA2 discharges north to the storm sewer system that is part of the Amira Apartments project.

The overall discharge rates from the proposed site will be less than the existing discharge rates in all events.

Discharge rates towards Drainage Area DA1 will be slightly higher than the existing rates heading this direction, but the storm sewer in this street appears to have been constructed to accept the full runoff from this site. The previously approved stormwater report for the Amira Apartments project (that included a hotel design for this site) had all of the runoff for this site directed to the storm sewer in Deer Creek Parkway.

The overall proposed runoff rates are less than the existing peak runoff rates. – REQUIREMENT SATISFIED

3.4 Volume Control

Retain 1.1 inches of runoff on-site from applicable impervious surfaces. If unique, site-specific constraints preclude infiltration, 60% TP and 90% TSS removal and no net increase in TP or TSS annually must be met.

Restricted Site Classification:

This site is not capable of meeting the required 1.1" abstraction and should be classified as restricted.

The MIDS flow chart asks the following questions to determine if a site is restricted:

Question 1 - "Are there very low infiltrating soils (<0.2 inches per hour)?"

Per the soils report, the existing soils are fill soils and very low permeability clay soils. These soils are classified with a hydrologic soil group "D" designation and not conducive to infiltration.

The Minnesota NPDES Construction Stormwater General Permit, which has requirements for permanent stormwater management systems, the permit severely limits infiltration in areas of Type "D" soils. For most projects in the state, the permit states (Ill.D.1.j.v.) "Infiltration is prohibited when the infiltration system will be constructed in: Areas of predominately Hydrologic Soil Group "D" (clay) soils

Question 2 - "Is BMP Relocation onsite to a lower-infiltrating location feasible?"

BMP relocation is not an option for this site, because the entire site has the same soils.

Question 3 - "Can BMP be sized to drain dry within 48 hours?"

An infiltration BMP is not appropriate for this site based on the low permeability soils present.

Question 4 - "Is FTO #1 (lower volume control standard) feasible, allowing the BMP to drain dry within 48 hours?"

FTO #1 is not feasible due to the soils present. An infiltration BMP cannot be designed to infiltrate 0.55" of runoff.

Based on these results, the MIDS flow chart indicates that FTO #2 be selected. FTO #2 has the following requirements:

- Achieve volume reduction to the maximum extent practicable.
- Remove 60% of the annual TP load.

Volume reduction is not feasible for this site due to the types of soils present. The MPCA NPDES permit does not allow infiltration when Hydrologic Soil Group "D" soils are present, which we would be on this site. Therefore, infiltration on this site is provided to the maximum extent practicable.

Water quality treatment will be provided in lieu of volume reduction per City of Minnetonka standards. –
REQUIREMENT SATISFIED

3.5 Water Quality

If unique, site-specific constraints preclude infiltration, 60% TP and 90% TSS removal and no net increase in TP or TSS annually must be met.

Based on the restrictive site classification discussion from above, this site needs to provide 60% TP and 90% TSS removal to meet the City of Minnetonka requirements.

Water quality is provided by the pretreatment systems and the underground detention pond. This underground stormwater pond has been designed to MPCA Level 3 Standards.

STORM WATER POND SIZING

Proposed BMP Area	Req. Vol (cf) Below Outlet (Dead Storage) 1800 cf/acre	Prov. Vol (cf) Below Outlet (Dead Storage)	Pond Surface Area (ac)	Req. MAX Disch. Rate (cfs) 5.66 cfs/acre [1.5" Rainfall]	Prov. Disch. Rate (cfs) [1.5" Rainfall Modeled]	Req. Vol (cf) Above Outlet (Live Storage) 1.5" x Impv. Area	Prov. Vol (cf) Above Outlet (Live Storage) 1.5" x Impv. Area
Underground Pond 1	4176	7467	0.10	0.57	0.57	8539	10775

*Underground Pond is designed to MPCA Level 3 Standards, which provides 60% TP and 90% TSS Removals.

*This project is providing abstraction to the maximum extent practicable. The site is restricted by site contamination and infiltration is not feasible.

The P8 Calculator has been utilized to verify that the project meets the TP and TSS reduction requirements. See the P8 Calculations for detailed analysis.

P8 CALCULATOR RESULTS

TOTAL PHOS. REMOVAL	60.5%
TSS REMOVAL	90.0%

NOTE: To model the pretreatment devices in P8, an arbitrary pond was developed that provided the same TSS removal that was calculated utilizing the Momentum Preserver Calculator.

Pretreatment TSS Removal per Momentum Preserver Calculator = 67.9%

Pretreatment TSS Removal in P8 Utilizing Arbitrary Pond = 66.8%

The proposed project will be reducing TP by 60.5% and TSS by 90.0%, meeting the City and watershed requirements.

4.0 Conclusions:

To the best of our knowledge, this project meets all State, City and Watershed District stormwater management requirements.

Amira Villas
Civil Site Group - Stormwater Calculations

Existing Conditions

Drainage Area	Impervious Area		Pervious Area		Total Area	
	Area [SF]	CN Value	Area [SF]	CN Value	Area [SF]	CN Value
EX1	9729	98	51485	80	61214	83
EX2	687	98	39156	80	39843	80

Proposed Conditions

Drainage Area	Impervious Area		Pervious Area		Total Area	
	Area [SF]	CN Value	Area [SF]	CN Value	Area [SF]	CN Value
PR1A	61334	98	12140	80	73474	95
PR1B	4181	98	10312	80	14493	85
PR2	2797	98	10293	80	13090	84

Site Area Summary

	Impervious [SF]	Impervious [AC]	Pervious [SF]	Pervious [AC]	Total [SF]	Total [AC]
Existing Site	10416	0.24	90641	2.08	101057	2.32
Proposed Site	68312	1.57	32745	0.75	101057	2.32

Stormwater Rate Summary

Drainage Area	Existing Rate (cfs)			
	1-YR [2.50"]	2-YR [2.88"]	10-YR [4.29"]	100-YR [7.42"]
EX1	2.76	3.50	6.43	13.28
EX2	1.54	2.01	3.91	8.39
TOTAL	4.30	5.51	10.34	21.67

Drainage Area	Proposed Conditions Rate (cfs)			
	1-YR [2.50"]	2-YR [2.88"]	10-YR [4.29"]	100-YR [7.42"]
1P (PR1A)	2.02	2.90	6.18	11.30
PR1B	0.74	0.92	1.61	3.23
PR2	0.62	0.78	1.41	2.87
TOTAL	3.38	4.60	9.20	17.40

Overall Stormwater Rate Summary

	Existing Conditions Rate (cfs)	Proposed Conditions Rate (cfs)
1-Year Event	4.30	3.38
2-Year Event	5.51	4.60
10-Year Event	10.34	9.20
100-Year Event	21.67	17.40

Stormwater Rate Summary - DA 1 - To Existing Storm Sewer South of Site

	Existing Conditions Rate (cfs)	Proposed Conditions Rate (cfs)
1-Year Event	2.76	2.76
2-Year Event	3.50	3.82
10-Year Event	6.43	7.79
100-Year Event	13.28	14.53

Stormwater Rate Summary - DA 2 - To Amira Apts North of Site

	Existing Conditions Rate (cfs)	Proposed Conditions Rate (cfs)
1-Year Event	1.54	0.62
2-Year Event	2.01	0.78
10-Year Event	3.91	1.41
100-Year Event	8.39	2.87

Stormwater Water Quality and Volume Summary

Drainage Area	Required Infiltration Vol. Summary		Infiltration Volume = 1.1*Dist. Impv. Area
	New Impv. Area (sf)	Required Volume (cf)	
PR1A	61334	10222	
PR1B	4181	697	
PR2	2797	466	
TOTAL	68312	11385	

STORM WATER POND SIZING

Proposed BMP Area	Req. Vol (cf) Below Outlet (Dead Storage) 1800 cf/acre	Prov. Vol (cf) Below Outlet (Dead Storage)	Pond Surface Area (ac)	Req. MAX Disch. Rate (cfs) 5.66 cfs/acre [1.5" Rainfall]	Prov. Disch. Rate (cfs) [1.5" Rainfall Modeled]	Req. Vol (cf) Above Outlet (Live Storage) 1.5" x Impv. Area	Prov. Vol (cf) Above Outlet (Live Storage) 1.5" x Impv. Area
Underground Pond 1	4176	7467	0.10	0.57	0.57	8539	10775

*Underground Pond is designed to MPCA Level 3 Standards, which provides 60% TP and 90% TSS Removals.

*This project is providing abstraction to the maximum extent practicable. The site is restricted by site contamination and infiltration is not feasible.

P8 CALCULATOR RESULTS

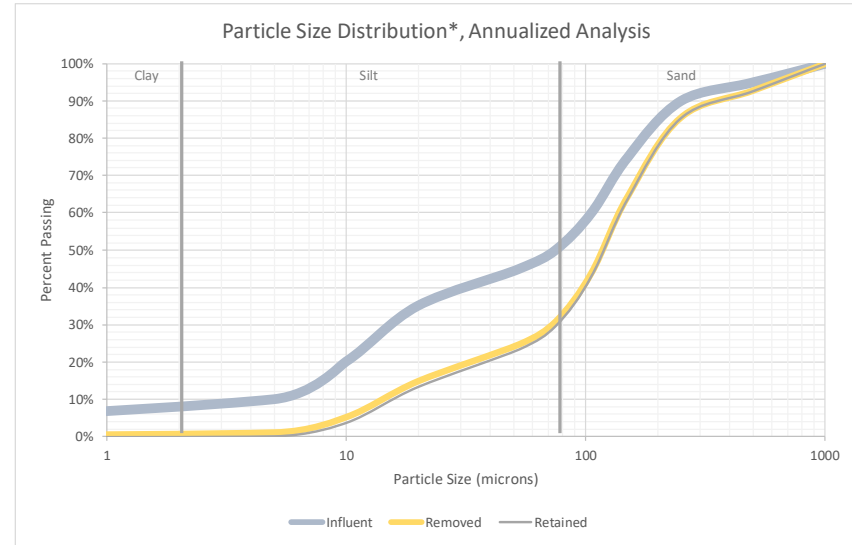
TOTAL PHOS. REMOVAL	60.5%
TSS REMOVAL	90.0%

Project:	AMIRA VILLAS
Location:	MINNETONKA
Structure ID:	SUMP STRUCTURES 11 & 21
Date:	6/4/2024

TREATMENT STRUCTURE SIZING CALCULATOR - VERSION 2.8

User Input:			
Variable	Value	Units	
Structure	Device ¹	Preserver, w/Skimmer	n/a
	Diameter	5	ft
	Sump Depth	5	ft
	Inlet Diameter	15	in
	Outlet Diameter	15	in
Hydrology	Location ²	MN, Twin Cities	n/a
	Drainage Area	0.84	acres
	Percent Impervious	81.9%	%
	Time of Concentration	10.0	minutes
	Pervious CN Value ³	80	unitless
	System Bypass?	No	n/a
	Water Temperature	68	°F
Pollutant Load	Particle Size Dist. ⁴	NJDEP	n/a
	Runoff Concentration	120	mg/l
	Specific Gravity	2.65	unitless

- 1) Refer to the "Devices" tab for device specifics (e.g. design and maintenance recommendations, etc.).
- 2) "Custom" rainfall data can be selected from the drop down list and entered below.
- 3) "Pervious CN Value" should include impervious areas not directly connected to the drainage system.
- 4) "Custom" Particle Size Distributions can be selected from the drop down list and entered below.



*This graph is accurate for particle size distributions only, not singular particle sizes.

Outputs:										
Annualized Pollutant Removal	Runoff Load (lbs.)	Bypassed Load (lbs.)	Load Removed, Excluding Scour (lbs.)	Removal Efficiency, Excluding Scour (%)	Scour Load (lbs.)	Load Removed, Including Scour (lbs.)	Removal Efficiency, Including Scour (%)	Pollutant Storage Depth (in)	Recommended Angle Between Pipes	Minimum Angle Between Pipes
	461		319	69.1%	6	313	67.9%	1.2	Future Version	

Device-Specific Design Assumptions → Pipe Invert Elevations Max. Pollutant Depth
(See "Devices" tab for details)

This calculator is downloadable at www.ThePreserver.com.

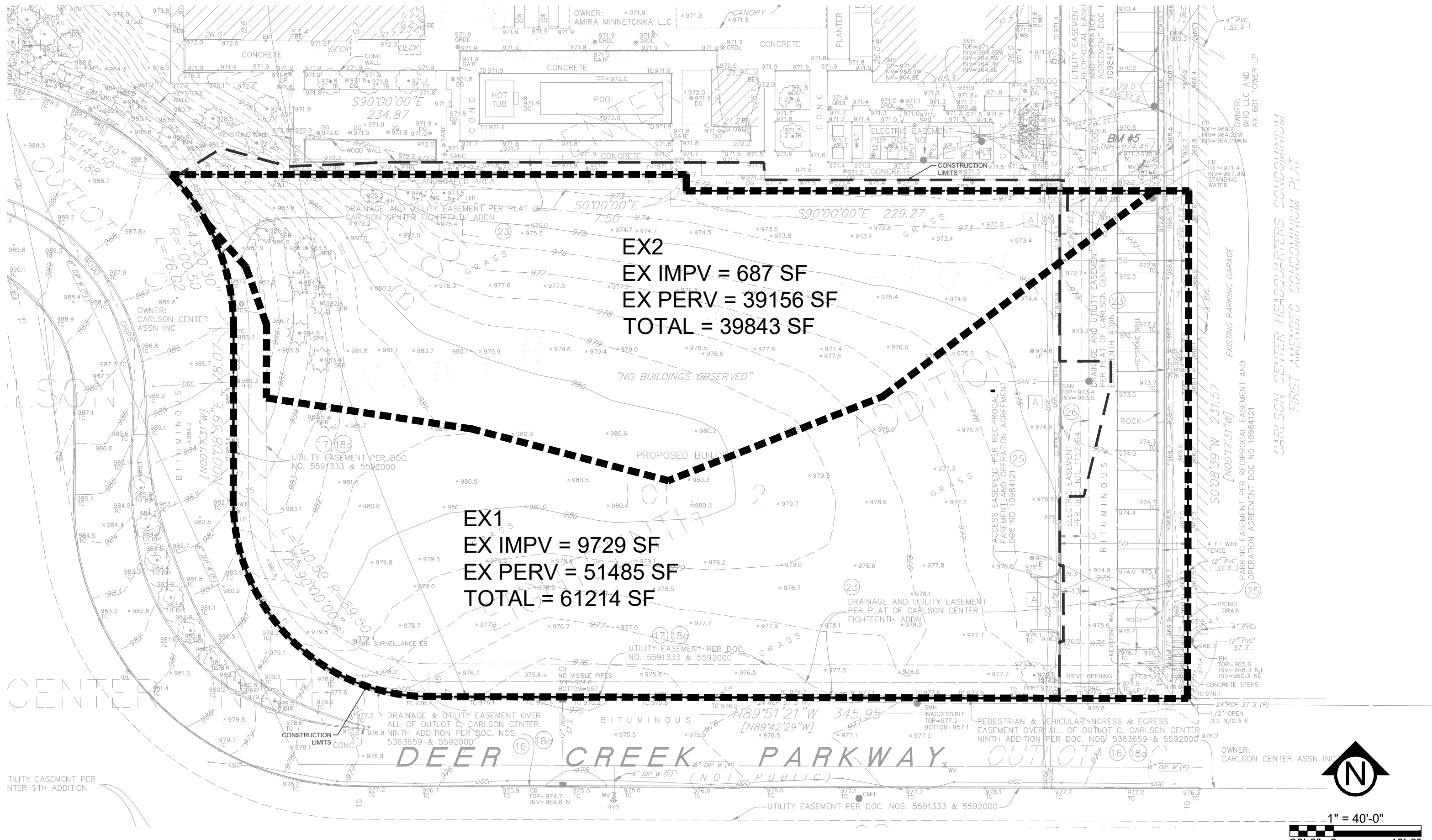
Copyright © 2024 Momentum Environmental LLC. All rights reserved.

P8 Model Results
 Amira Villas Minnetonka
 6/7/24
 PERCENT REMOVAL

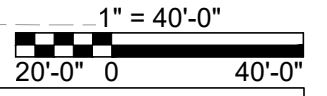
Variable	OVERALL	LAWN AREAS	PRETREATMENT STRUCTURES - WITH MOMENTUM PRESERVER	UNDERGROUND STORMWATER POND	CULVERT
P0%					
P10%	69.2	33.2	30.8	63.6	
P30%	86.1	70.7	48.7	77.6	
P50%	95.4	87.9	66.5	89.8	
P80%	99.7	98.2	93.4	99.0	
TSS	90.0	77.6	66.6	75.9	
TP	60.5	46.8	35.2	42.4	
TKN	52.5	40.8	30.5	34.2	
CU	66.4	57.9	49.0	36.7	
PB	81.9	70.9	60.5	58.6	
ZN	52.5	40.8	30.5	34.2	
HC	81.9	70.9	60.5	58.6	

LBS REMOVED PER YEAR

Variable	OVERALL	LAWN AREAS	PRETREATMENT STRUCTURES - WITH MOMENTUM PRESERVER	UNDERGROUND STORMWATER POND	CULVERT
P0%					
P10%	153.0	9.9	59.0	84.1	
P30%	190.4	21.2	93.1	76.2	
P50%	211.1	26.4	127.2	57.5	
P80%	441.2	58.9	357.1	25.2	
TSS	995.7	116.4	636.5	242.9	
TP	2.1	0.2	1.1	0.8	
TKN	8.3	0.9	4.2	3.3	
CU	0.3	0.0	0.2	0.1	
PB	0.2	0.0	0.1	0.0	
ZN	0.9	0.1	0.4	0.3	
HC	22.4	2.6	14.3	5.5	



CARLSON CENTER HEADQUARTERS CONDOMINIUM
 FIRST AMENDED CONDOMINIUM PLAN



AMIRA VILLAS

EXISTING DRAINAGE MAP

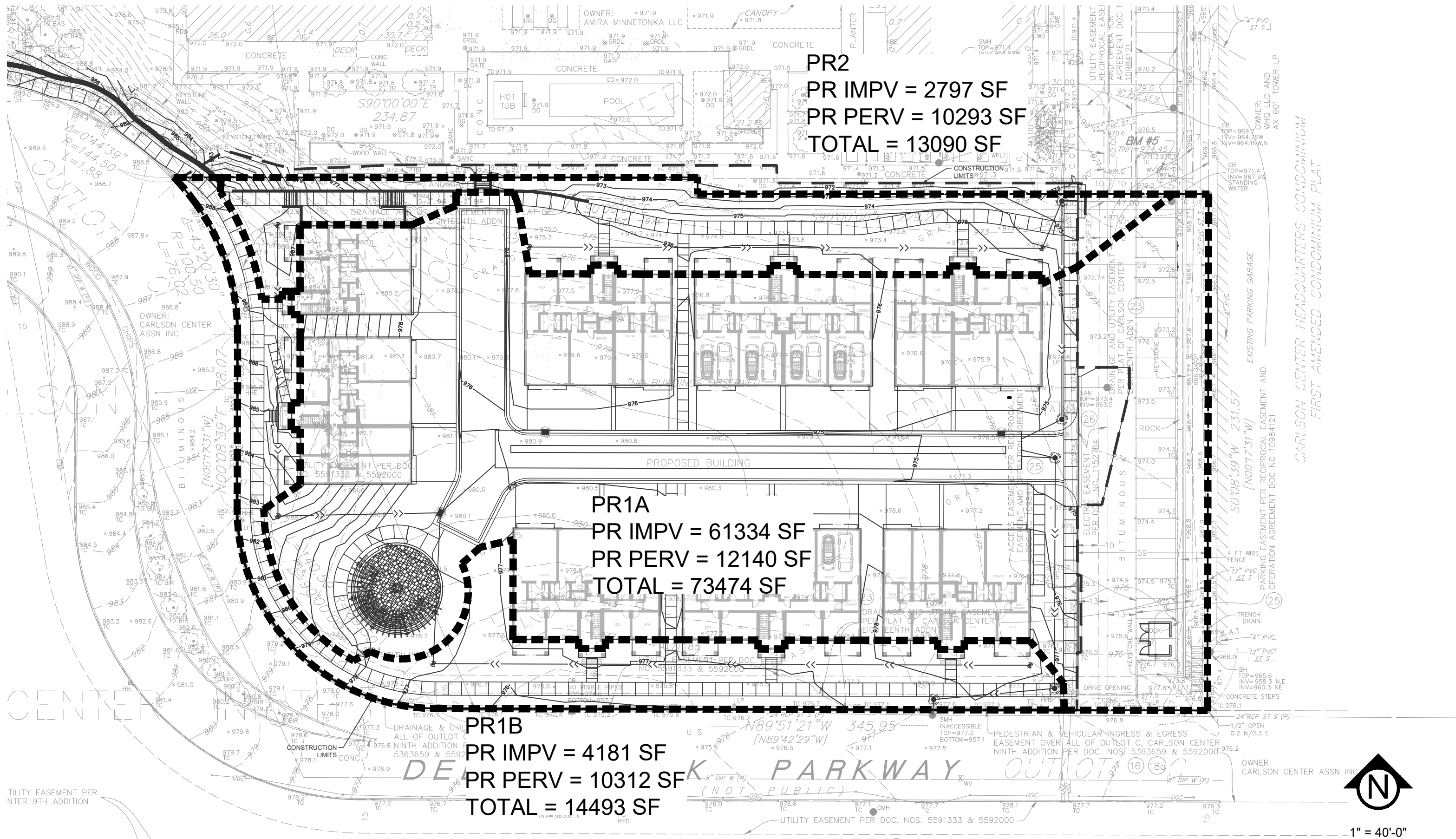
901 CARLSON PARKWAY, MINNETONKA, MN 55305

CivilSite
 GROUP

5000 Glenwood Avenue
 Golden Valley, MN 55422
 612-615-0060
 www.CivilSiteGroup.com

Project Number:	24132	Revision Number:	
Issue Date:	6/7/24	Revision Date:	

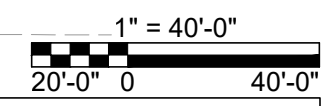
DA 1



PR2
PR IMPV = 2797 SF
PR PERV = 10293 SF
TOTAL = 13090 SF

PR1A
PR IMPV = 61334 SF
PR PERV = 12140 SF
TOTAL = 73474 SF

PR1B
PR IMPV = 4181 SF
PR PERV = 10312 SF
TOTAL = 14493 SF



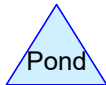
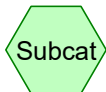
EXISTING CONDITIONS



DRAINS NORTH TO
AMIRA APTS SITE



DRAINS SOUTH TO
EX. STORM SEWER



24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Printed 6/5/2024

Page 2

Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1y 24hr AT-14	MSE 24-hr	3	Default	24.00	1	2.50	2
2	2y 24hr AT-14	MSE 24-hr	3	Default	24.00	1	2.88	2
3	10y 24hr AT-14	MSE 24-hr	3	Default	24.00	1	4.29	2
4	100y 24hr AT-14	MSE 24-hr	3	Default	24.00	1	7.42	2

24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Printed 6/5/2024

Page 3

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
2.081	80	>75% Grass cover, Good, HSG D (EX1, EX2)
0.239	98	Paved parking, HSG D (EX1, EX2)
2.320	82	TOTAL AREA

24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Printed 6/5/2024

Page 4

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
2.320	HSG D	EX1, EX2
0.000	Other	
2.320		TOTAL AREA

24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Printed 6/5/2024

Page 5

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	2.081	0.000	2.081	>75% Grass cover, Good	EX1, EX2
0.000	0.000	0.000	0.239	0.000	0.239	Paved parking	EX1, EX2
0.000	0.000	0.000	2.320	0.000	2.320	TOTAL AREA	

24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Printed 6/5/2024

Page 7

Summary for Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Runoff = 2.76 cfs @ 12.14 hrs, Volume= 0.130 af, Depth= 1.11"
Routed to nonexistent node EX-DA 1

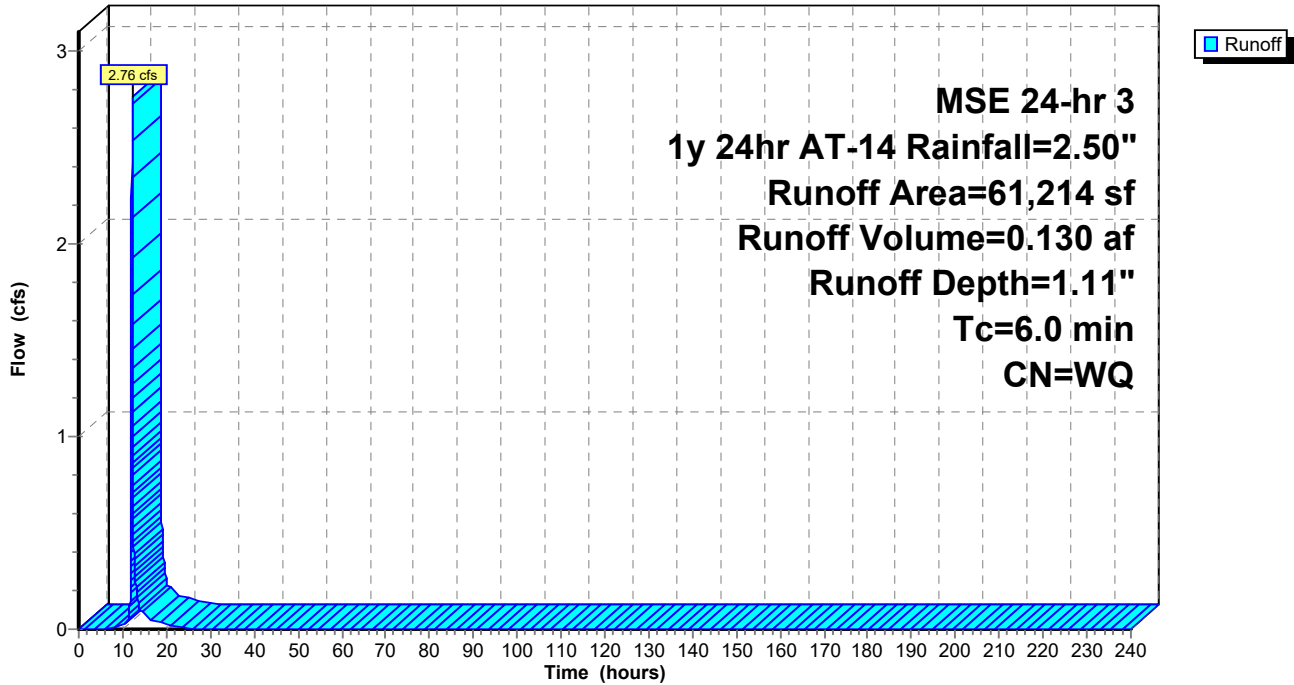
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Area (sf)	CN	Description
9,729	98	Paved parking, HSG D
51,485	80	>75% Grass cover, Good, HSG D
61,214		Weighted Average
51,485		84.11% Pervious Area
9,729		15.89% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Hydrograph



24132 EXISTING*MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"*

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 8

Hydrograph for Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.09	0.00	0.00
10.00	0.34	0.00	0.02
15.00	2.24	0.87	0.08
20.00	2.43	1.01	0.03
25.00	2.50	1.06	0.00
30.00	2.50	1.06	0.00
35.00	2.50	1.06	0.00
40.00	2.50	1.06	0.00
45.00	2.50	1.06	0.00
50.00	2.50	1.06	0.00
55.00	2.50	1.06	0.00
60.00	2.50	1.06	0.00
65.00	2.50	1.06	0.00
70.00	2.50	1.06	0.00
75.00	2.50	1.06	0.00
80.00	2.50	1.06	0.00
85.00	2.50	1.06	0.00
90.00	2.50	1.06	0.00
95.00	2.50	1.06	0.00
100.00	2.50	1.06	0.00
105.00	2.50	1.06	0.00
110.00	2.50	1.06	0.00
115.00	2.50	1.06	0.00
120.00	2.50	1.06	0.00
125.00	2.50	1.06	0.00
130.00	2.50	1.06	0.00
135.00	2.50	1.06	0.00
140.00	2.50	1.06	0.00
145.00	2.50	1.06	0.00
150.00	2.50	1.06	0.00
155.00	2.50	1.06	0.00
160.00	2.50	1.06	0.00
165.00	2.50	1.06	0.00
170.00	2.50	1.06	0.00
175.00	2.50	1.06	0.00
180.00	2.50	1.06	0.00
185.00	2.50	1.06	0.00
190.00	2.50	1.06	0.00
195.00	2.50	1.06	0.00
200.00	2.50	1.06	0.00
205.00	2.50	1.06	0.00
210.00	2.50	1.06	0.00
215.00	2.50	1.06	0.00
220.00	2.50	1.06	0.00
225.00	2.50	1.06	0.00
230.00	2.50	1.06	0.00
235.00	2.50	1.06	0.00
240.00	2.50	1.06	0.00

24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Printed 6/5/2024

Page 9

Summary for Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Runoff = 1.54 cfs @ 12.14 hrs, Volume= 0.070 af, Depth= 0.91"
 Routed to nonexistent node EX-DA 1

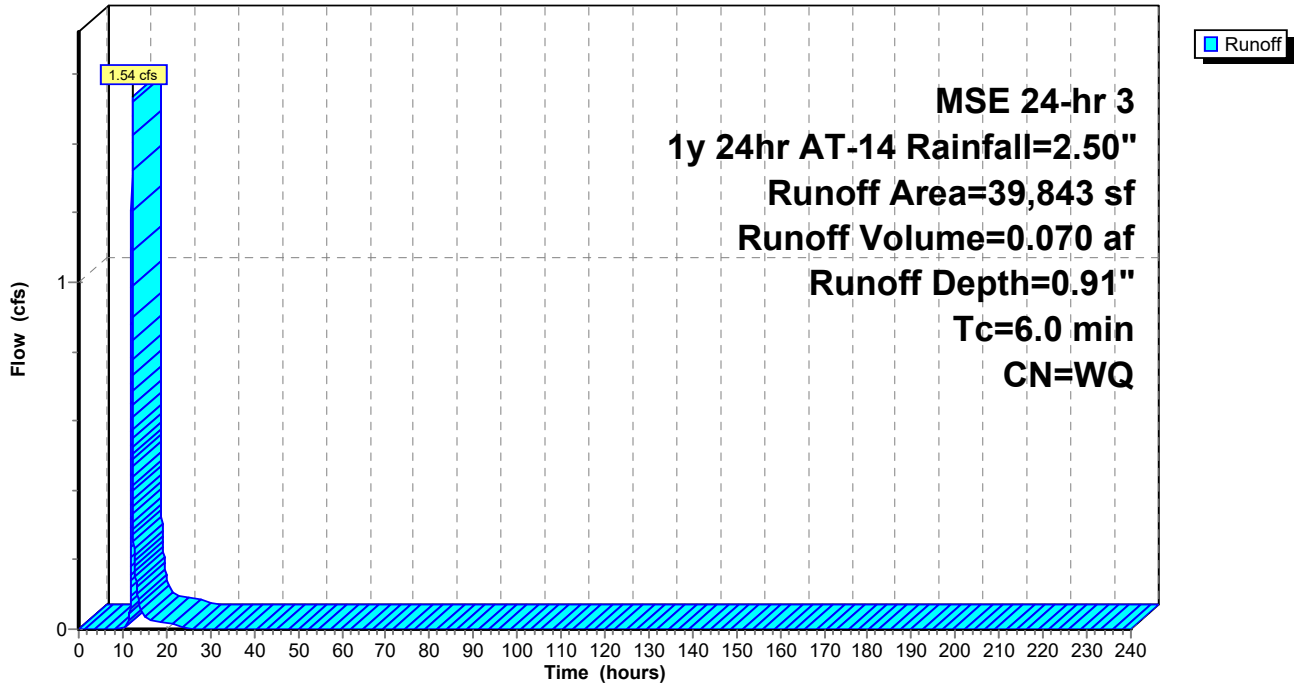
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Area (sf)	CN	Description
687	98	Paved parking, HSG D
39,156	80	>75% Grass cover, Good, HSG D
39,843		Weighted Average
39,156		98.28% Pervious Area
687		1.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Hydrograph



24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Printed 6/5/2024

Page 10

Hydrograph for Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.09	0.00	0.00
10.00	0.34	0.00	0.00
15.00	2.24	0.72	0.05
20.00	2.43	0.84	0.02
25.00	2.50	0.89	0.00
30.00	2.50	0.89	0.00
35.00	2.50	0.89	0.00
40.00	2.50	0.89	0.00
45.00	2.50	0.89	0.00
50.00	2.50	0.89	0.00
55.00	2.50	0.89	0.00
60.00	2.50	0.89	0.00
65.00	2.50	0.89	0.00
70.00	2.50	0.89	0.00
75.00	2.50	0.89	0.00
80.00	2.50	0.89	0.00
85.00	2.50	0.89	0.00
90.00	2.50	0.89	0.00
95.00	2.50	0.89	0.00
100.00	2.50	0.89	0.00
105.00	2.50	0.89	0.00
110.00	2.50	0.89	0.00
115.00	2.50	0.89	0.00
120.00	2.50	0.89	0.00
125.00	2.50	0.89	0.00
130.00	2.50	0.89	0.00
135.00	2.50	0.89	0.00
140.00	2.50	0.89	0.00
145.00	2.50	0.89	0.00
150.00	2.50	0.89	0.00
155.00	2.50	0.89	0.00
160.00	2.50	0.89	0.00
165.00	2.50	0.89	0.00
170.00	2.50	0.89	0.00
175.00	2.50	0.89	0.00
180.00	2.50	0.89	0.00
185.00	2.50	0.89	0.00
190.00	2.50	0.89	0.00
195.00	2.50	0.89	0.00
200.00	2.50	0.89	0.00
205.00	2.50	0.89	0.00
210.00	2.50	0.89	0.00
215.00	2.50	0.89	0.00
220.00	2.50	0.89	0.00
225.00	2.50	0.89	0.00
230.00	2.50	0.89	0.00
235.00	2.50	0.89	0.00
240.00	2.50	0.89	0.00

24132 EXISTING

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 11

Time span=0.00-240.00 hrs, dt=0.01 hrs, 24001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

SubcatchmentEX1: DRAINS SOUTH TO Runoff Area=61,214 sf 15.89% Impervious Runoff Depth=1.40"
Tc=6.0 min CN=WQ Runoff=3.50 cfs 0.164 af

SubcatchmentEX2: DRAINS NORTH TO Runoff Area=39,843 sf 1.72% Impervious Runoff Depth=1.19"
Tc=6.0 min CN=WQ Runoff=2.01 cfs 0.090 af

Total Runoff Area = 2.320 ac Runoff Volume = 0.254 af Average Runoff Depth = 1.31"
89.69% Pervious = 2.081 ac 10.31% Impervious = 0.239 ac

24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 12

Summary for Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Runoff = 3.50 cfs @ 12.13 hrs, Volume= 0.164 af, Depth= 1.40"
Routed to nonexistent node EX-DA 1

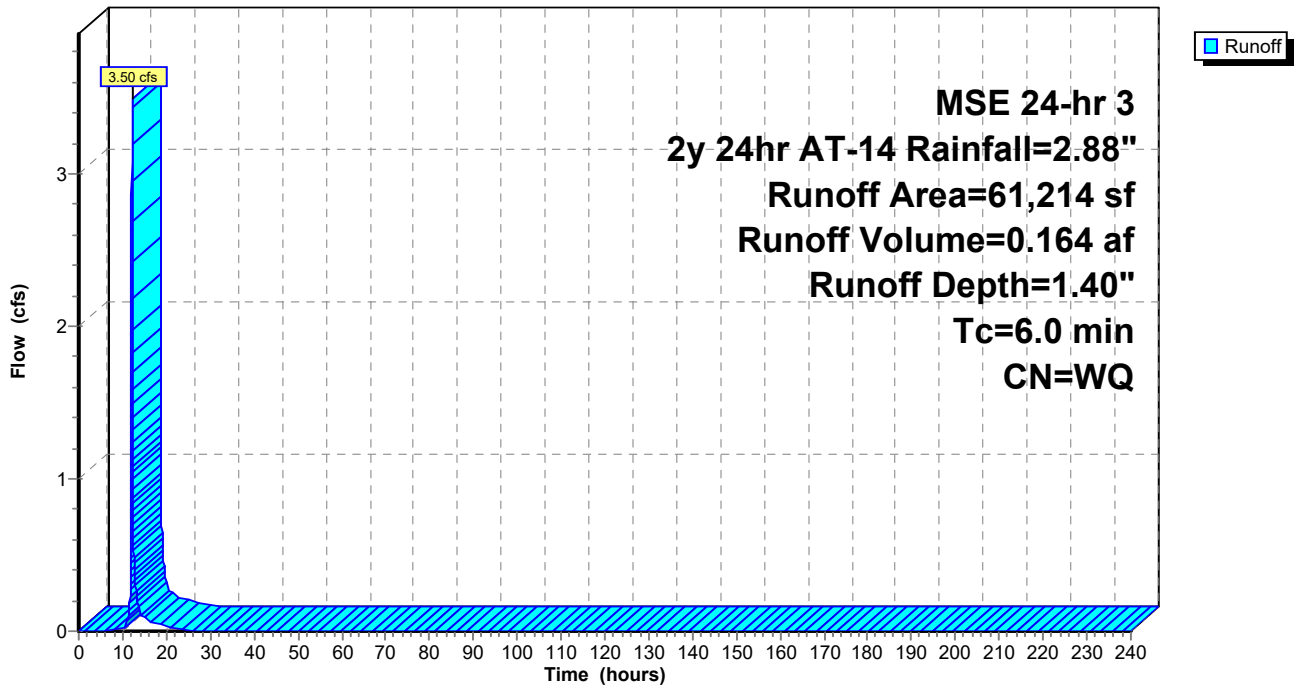
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Area (sf)	CN	Description
9,729	98	Paved parking, HSG D
51,485	80	>75% Grass cover, Good, HSG D
61,214		Weighted Average
51,485		84.11% Pervious Area
9,729		15.89% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Hydrograph



24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 13

Hydrograph for Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.11	0.00	0.00
10.00	0.40	0.00	0.02
15.00	2.58	1.12	0.10
20.00	2.80	1.29	0.03
25.00	2.88	1.35	0.00
30.00	2.88	1.35	0.00
35.00	2.88	1.35	0.00
40.00	2.88	1.35	0.00
45.00	2.88	1.35	0.00
50.00	2.88	1.35	0.00
55.00	2.88	1.35	0.00
60.00	2.88	1.35	0.00
65.00	2.88	1.35	0.00
70.00	2.88	1.35	0.00
75.00	2.88	1.35	0.00
80.00	2.88	1.35	0.00
85.00	2.88	1.35	0.00
90.00	2.88	1.35	0.00
95.00	2.88	1.35	0.00
100.00	2.88	1.35	0.00
105.00	2.88	1.35	0.00
110.00	2.88	1.35	0.00
115.00	2.88	1.35	0.00
120.00	2.88	1.35	0.00
125.00	2.88	1.35	0.00
130.00	2.88	1.35	0.00
135.00	2.88	1.35	0.00
140.00	2.88	1.35	0.00
145.00	2.88	1.35	0.00
150.00	2.88	1.35	0.00
155.00	2.88	1.35	0.00
160.00	2.88	1.35	0.00
165.00	2.88	1.35	0.00
170.00	2.88	1.35	0.00
175.00	2.88	1.35	0.00
180.00	2.88	1.35	0.00
185.00	2.88	1.35	0.00
190.00	2.88	1.35	0.00
195.00	2.88	1.35	0.00
200.00	2.88	1.35	0.00
205.00	2.88	1.35	0.00
210.00	2.88	1.35	0.00
215.00	2.88	1.35	0.00
220.00	2.88	1.35	0.00
225.00	2.88	1.35	0.00
230.00	2.88	1.35	0.00
235.00	2.88	1.35	0.00
240.00	2.88	1.35	0.00

24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 14

Summary for Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Runoff = 2.01 cfs @ 12.14 hrs, Volume= 0.090 af, Depth= 1.19"
 Routed to nonexistent node EX-DA 1

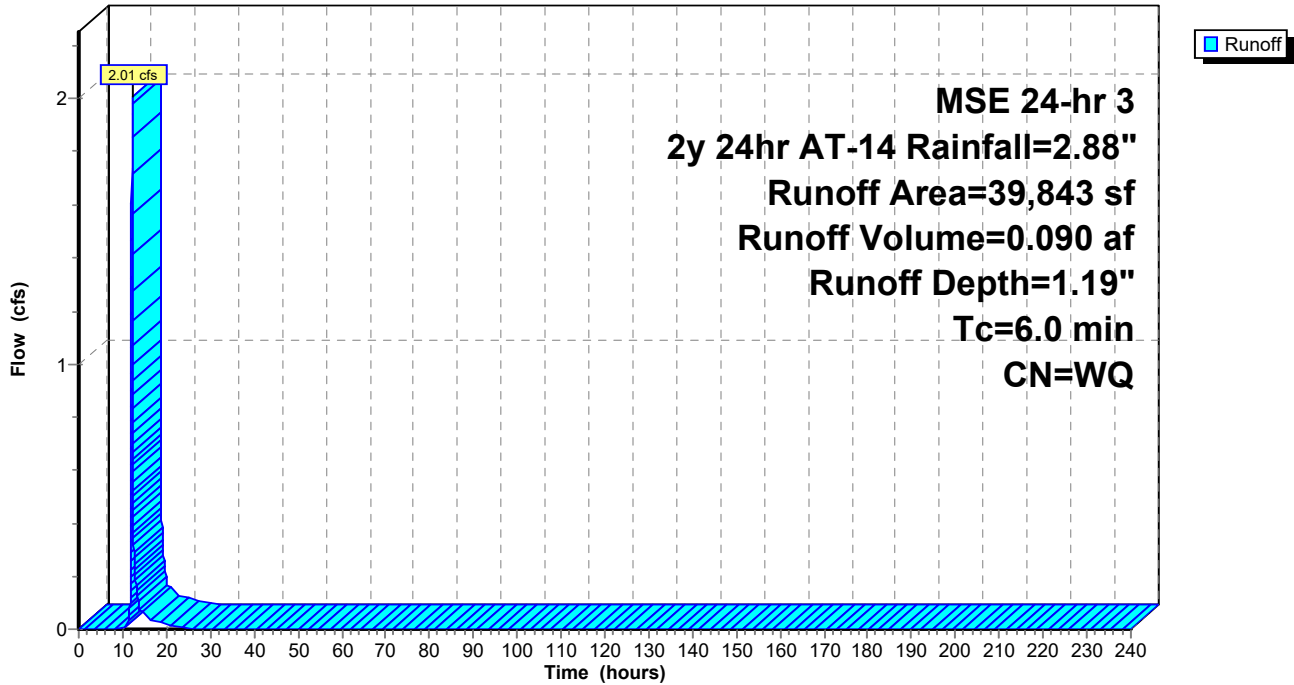
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Area (sf)	CN	Description
687	98	Paved parking, HSG D
39,156	80	>75% Grass cover, Good, HSG D
39,843		Weighted Average
39,156		98.28% Pervious Area
687		1.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Hydrograph



24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 15

Hydrograph for Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.11	0.00	0.00
10.00	0.40	0.00	0.00
15.00	2.58	0.95	0.06
20.00	2.80	1.11	0.02
25.00	2.88	1.16	0.00
30.00	2.88	1.16	0.00
35.00	2.88	1.16	0.00
40.00	2.88	1.16	0.00
45.00	2.88	1.16	0.00
50.00	2.88	1.16	0.00
55.00	2.88	1.16	0.00
60.00	2.88	1.16	0.00
65.00	2.88	1.16	0.00
70.00	2.88	1.16	0.00
75.00	2.88	1.16	0.00
80.00	2.88	1.16	0.00
85.00	2.88	1.16	0.00
90.00	2.88	1.16	0.00
95.00	2.88	1.16	0.00
100.00	2.88	1.16	0.00
105.00	2.88	1.16	0.00
110.00	2.88	1.16	0.00
115.00	2.88	1.16	0.00
120.00	2.88	1.16	0.00
125.00	2.88	1.16	0.00
130.00	2.88	1.16	0.00
135.00	2.88	1.16	0.00
140.00	2.88	1.16	0.00
145.00	2.88	1.16	0.00
150.00	2.88	1.16	0.00
155.00	2.88	1.16	0.00
160.00	2.88	1.16	0.00
165.00	2.88	1.16	0.00
170.00	2.88	1.16	0.00
175.00	2.88	1.16	0.00
180.00	2.88	1.16	0.00
185.00	2.88	1.16	0.00
190.00	2.88	1.16	0.00
195.00	2.88	1.16	0.00
200.00	2.88	1.16	0.00
205.00	2.88	1.16	0.00
210.00	2.88	1.16	0.00
215.00	2.88	1.16	0.00
220.00	2.88	1.16	0.00
225.00	2.88	1.16	0.00
230.00	2.88	1.16	0.00
235.00	2.88	1.16	0.00
240.00	2.88	1.16	0.00

24132 EXISTING

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 16

Time span=0.00-240.00 hrs, dt=0.01 hrs, 24001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

SubcatchmentEX1: DRAINS SOUTH TO Runoff Area=61,214 sf 15.89% Impervious Runoff Depth=2.57"
Tc=6.0 min CN=WQ Runoff=6.43 cfs 0.300 af

SubcatchmentEX2: DRAINS NORTH TO Runoff Area=39,843 sf 1.72% Impervious Runoff Depth=2.31"
Tc=6.0 min CN=WQ Runoff=3.91 cfs 0.176 af

Total Runoff Area = 2.320 ac Runoff Volume = 0.477 af Average Runoff Depth = 2.47"
89.69% Pervious = 2.081 ac 10.31% Impervious = 0.239 ac

24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 17

Summary for Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Runoff = 6.43 cfs @ 12.13 hrs, Volume= 0.300 af, Depth= 2.57"
Routed to nonexistent node EX-DA 1

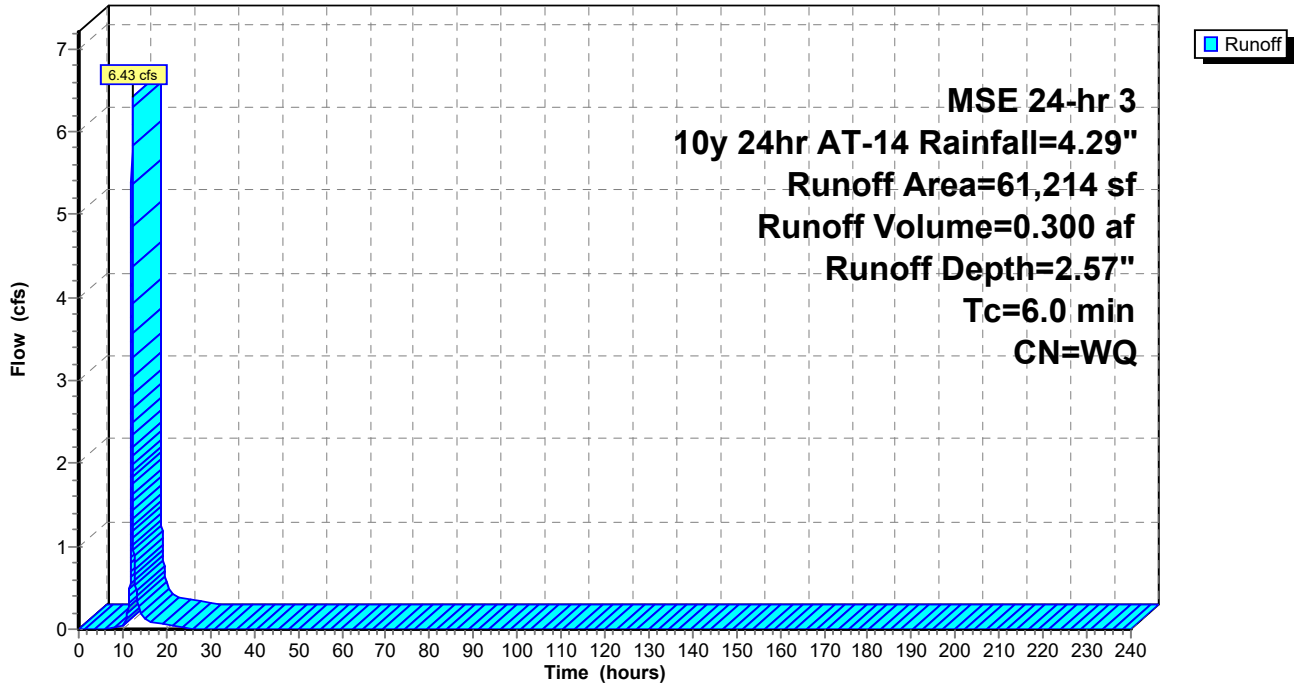
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Area (sf)	CN	Description
9,729	98	Paved parking, HSG D
51,485	80	>75% Grass cover, Good, HSG D
61,214		Weighted Average
51,485		84.11% Pervious Area
9,729		15.89% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Hydrograph



24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 18

Hydrograph for Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.16	0.00	0.01
10.00	0.59	0.01	0.04
15.00	3.85	2.15	0.17
20.00	4.18	2.44	0.06
25.00	4.29	2.54	0.00
30.00	4.29	2.54	0.00
35.00	4.29	2.54	0.00
40.00	4.29	2.54	0.00
45.00	4.29	2.54	0.00
50.00	4.29	2.54	0.00
55.00	4.29	2.54	0.00
60.00	4.29	2.54	0.00
65.00	4.29	2.54	0.00
70.00	4.29	2.54	0.00
75.00	4.29	2.54	0.00
80.00	4.29	2.54	0.00
85.00	4.29	2.54	0.00
90.00	4.29	2.54	0.00
95.00	4.29	2.54	0.00
100.00	4.29	2.54	0.00
105.00	4.29	2.54	0.00
110.00	4.29	2.54	0.00
115.00	4.29	2.54	0.00
120.00	4.29	2.54	0.00
125.00	4.29	2.54	0.00
130.00	4.29	2.54	0.00
135.00	4.29	2.54	0.00
140.00	4.29	2.54	0.00
145.00	4.29	2.54	0.00
150.00	4.29	2.54	0.00
155.00	4.29	2.54	0.00
160.00	4.29	2.54	0.00
165.00	4.29	2.54	0.00
170.00	4.29	2.54	0.00
175.00	4.29	2.54	0.00
180.00	4.29	2.54	0.00
185.00	4.29	2.54	0.00
190.00	4.29	2.54	0.00
195.00	4.29	2.54	0.00
200.00	4.29	2.54	0.00
205.00	4.29	2.54	0.00
210.00	4.29	2.54	0.00
215.00	4.29	2.54	0.00
220.00	4.29	2.54	0.00
225.00	4.29	2.54	0.00
230.00	4.29	2.54	0.00
235.00	4.29	2.54	0.00
240.00	4.29	2.54	0.00

24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 19

Summary for Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Runoff = 3.91 cfs @ 12.13 hrs, Volume= 0.176 af, Depth= 2.31"
Routed to nonexistent node EX-DA 1

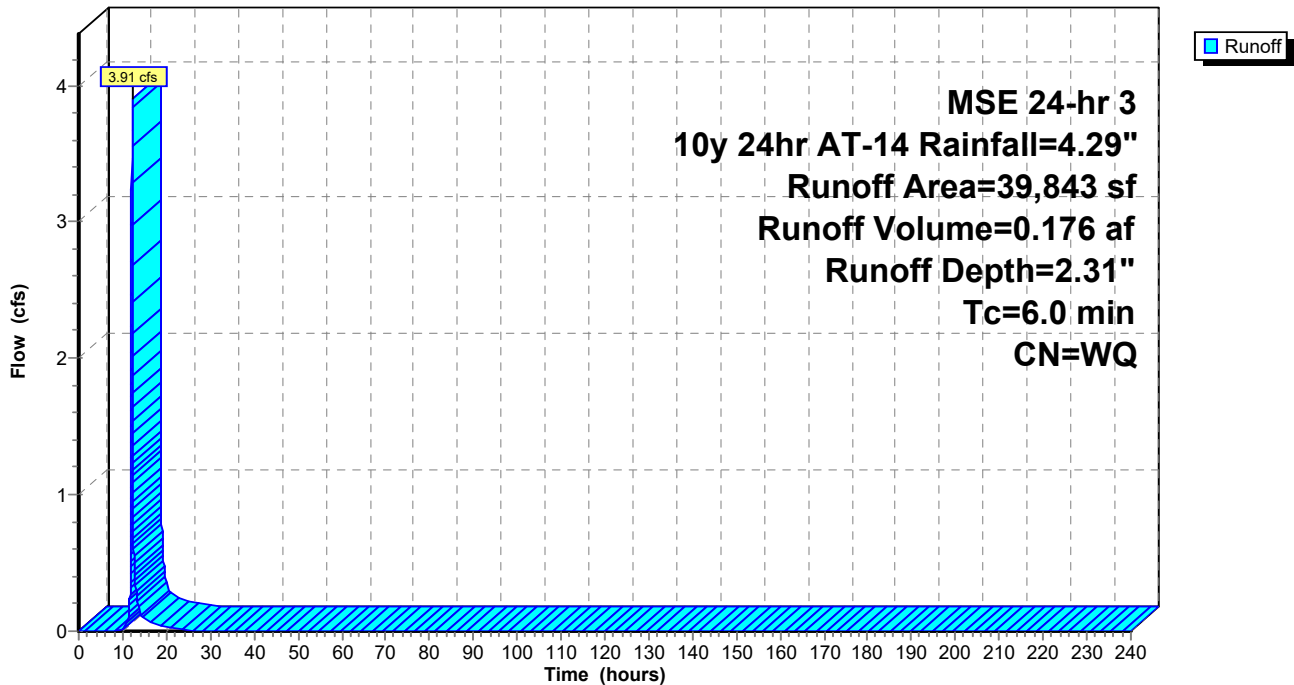
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Area (sf)	CN	Description
687	98	Paved parking, HSG D
39,156	80	>75% Grass cover, Good, HSG D
39,843		Weighted Average
39,156		98.28% Pervious Area
687		1.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Hydrograph



24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 20

Hydrograph for Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.16	0.00	0.00
10.00	0.59	0.00	0.01
15.00	3.85	1.92	0.11
20.00	4.18	2.19	0.04
25.00	4.29	2.28	0.00
30.00	4.29	2.28	0.00
35.00	4.29	2.28	0.00
40.00	4.29	2.28	0.00
45.00	4.29	2.28	0.00
50.00	4.29	2.28	0.00
55.00	4.29	2.28	0.00
60.00	4.29	2.28	0.00
65.00	4.29	2.28	0.00
70.00	4.29	2.28	0.00
75.00	4.29	2.28	0.00
80.00	4.29	2.28	0.00
85.00	4.29	2.28	0.00
90.00	4.29	2.28	0.00
95.00	4.29	2.28	0.00
100.00	4.29	2.28	0.00
105.00	4.29	2.28	0.00
110.00	4.29	2.28	0.00
115.00	4.29	2.28	0.00
120.00	4.29	2.28	0.00
125.00	4.29	2.28	0.00
130.00	4.29	2.28	0.00
135.00	4.29	2.28	0.00
140.00	4.29	2.28	0.00
145.00	4.29	2.28	0.00
150.00	4.29	2.28	0.00
155.00	4.29	2.28	0.00
160.00	4.29	2.28	0.00
165.00	4.29	2.28	0.00
170.00	4.29	2.28	0.00
175.00	4.29	2.28	0.00
180.00	4.29	2.28	0.00
185.00	4.29	2.28	0.00
190.00	4.29	2.28	0.00
195.00	4.29	2.28	0.00
200.00	4.29	2.28	0.00
205.00	4.29	2.28	0.00
210.00	4.29	2.28	0.00
215.00	4.29	2.28	0.00
220.00	4.29	2.28	0.00
225.00	4.29	2.28	0.00
230.00	4.29	2.28	0.00
235.00	4.29	2.28	0.00
240.00	4.29	2.28	0.00

24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 22

Summary for Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Runoff = 13.28 cfs @ 12.13 hrs, Volume= 0.634 af, Depth= 5.42"
Routed to nonexistent node EX-DA 1

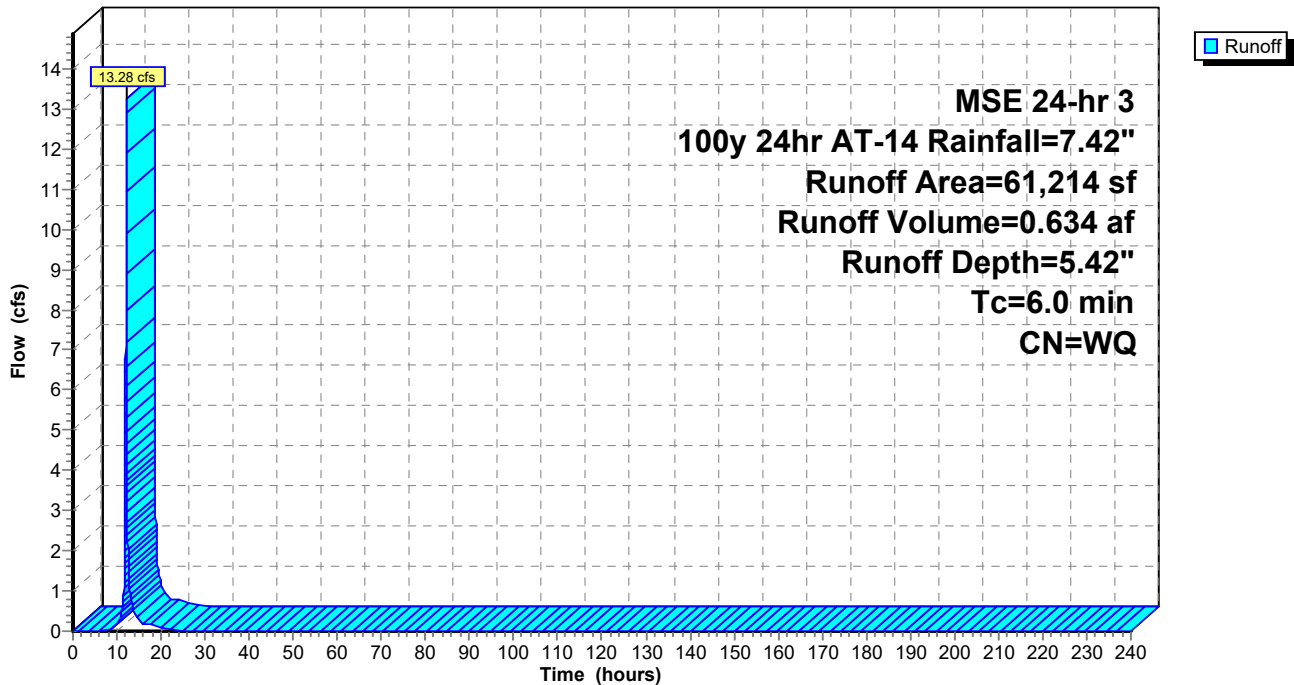
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Area (sf)	CN	Description
9,729	98	Paved parking, HSG D
51,485	80	>75% Grass cover, Good, HSG D
61,214		Weighted Average
51,485		84.11% Pervious Area
9,729		15.89% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Hydrograph



24132 EXISTING*MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"*

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

Page 23

Hydrograph for Subcatchment EX1: DRAINS SOUTH TO EX. STORM SEWER

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.28	0.00	0.02
10.00	1.02	0.14	0.16
15.00	6.66	4.70	0.32
20.00	7.23	5.24	0.10
25.00	7.42	5.43	0.00
30.00	7.42	5.43	0.00
35.00	7.42	5.43	0.00
40.00	7.42	5.43	0.00
45.00	7.42	5.43	0.00
50.00	7.42	5.43	0.00
55.00	7.42	5.43	0.00
60.00	7.42	5.43	0.00
65.00	7.42	5.43	0.00
70.00	7.42	5.43	0.00
75.00	7.42	5.43	0.00
80.00	7.42	5.43	0.00
85.00	7.42	5.43	0.00
90.00	7.42	5.43	0.00
95.00	7.42	5.43	0.00
100.00	7.42	5.43	0.00
105.00	7.42	5.43	0.00
110.00	7.42	5.43	0.00
115.00	7.42	5.43	0.00
120.00	7.42	5.43	0.00
125.00	7.42	5.43	0.00
130.00	7.42	5.43	0.00
135.00	7.42	5.43	0.00
140.00	7.42	5.43	0.00
145.00	7.42	5.43	0.00
150.00	7.42	5.43	0.00
155.00	7.42	5.43	0.00
160.00	7.42	5.43	0.00
165.00	7.42	5.43	0.00
170.00	7.42	5.43	0.00
175.00	7.42	5.43	0.00
180.00	7.42	5.43	0.00
185.00	7.42	5.43	0.00
190.00	7.42	5.43	0.00
195.00	7.42	5.43	0.00
200.00	7.42	5.43	0.00
205.00	7.42	5.43	0.00
210.00	7.42	5.43	0.00
215.00	7.42	5.43	0.00
220.00	7.42	5.43	0.00
225.00	7.42	5.43	0.00
230.00	7.42	5.43	0.00
235.00	7.42	5.43	0.00
240.00	7.42	5.43	0.00

24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 24

Summary for Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Runoff = 8.39 cfs @ 12.13 hrs, Volume= 0.390 af, Depth= 5.12"
 Routed to nonexistent node EX-DA 1

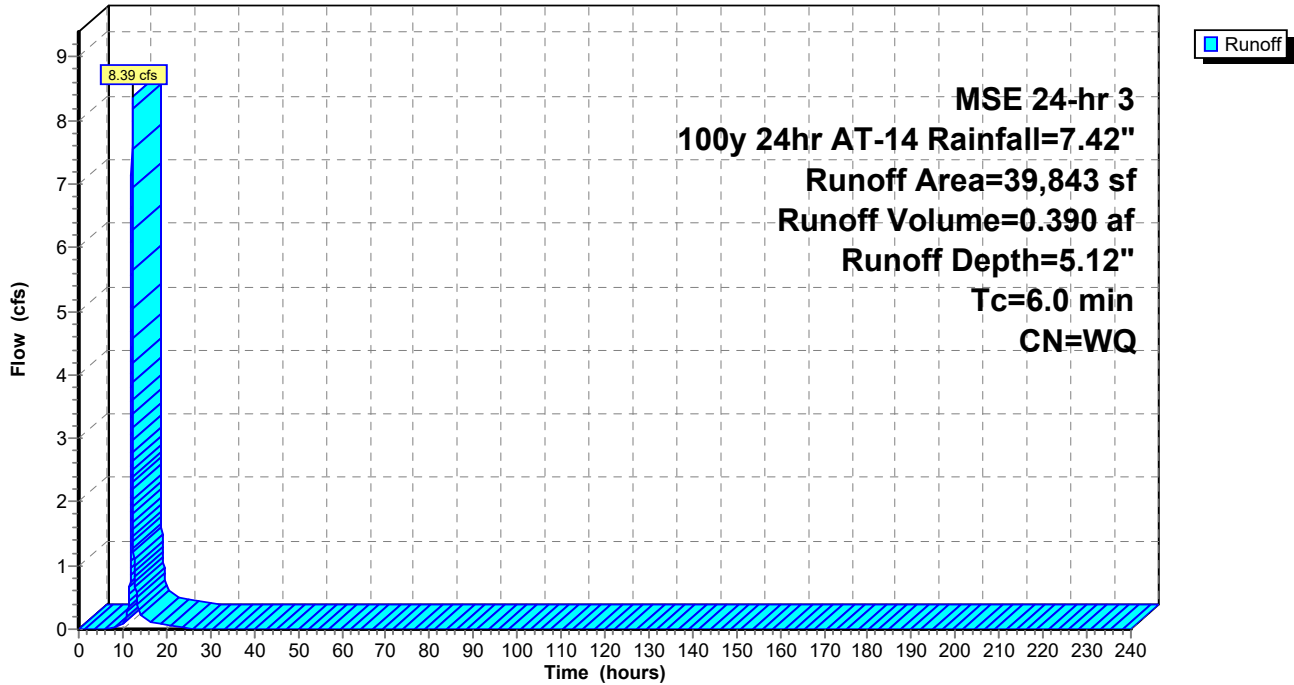
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Area (sf)	CN	Description
687	98	Paved parking, HSG D
39,156	80	>75% Grass cover, Good, HSG D
39,843		Weighted Average
39,156		98.28% Pervious Area
687		1.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Hydrograph



24132 EXISTING

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 25

Hydrograph for Subcatchment EX2: DRAINS NORTH TO AMIRA APTS SITE

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.28	0.00	0.00
10.00	1.02	0.09	0.08
15.00	6.66	4.38	0.20
20.00	7.23	4.90	0.07
25.00	7.42	5.08	0.00
30.00	7.42	5.08	0.00
35.00	7.42	5.08	0.00
40.00	7.42	5.08	0.00
45.00	7.42	5.08	0.00
50.00	7.42	5.08	0.00
55.00	7.42	5.08	0.00
60.00	7.42	5.08	0.00
65.00	7.42	5.08	0.00
70.00	7.42	5.08	0.00
75.00	7.42	5.08	0.00
80.00	7.42	5.08	0.00
85.00	7.42	5.08	0.00
90.00	7.42	5.08	0.00
95.00	7.42	5.08	0.00
100.00	7.42	5.08	0.00
105.00	7.42	5.08	0.00
110.00	7.42	5.08	0.00
115.00	7.42	5.08	0.00
120.00	7.42	5.08	0.00
125.00	7.42	5.08	0.00
130.00	7.42	5.08	0.00
135.00	7.42	5.08	0.00
140.00	7.42	5.08	0.00
145.00	7.42	5.08	0.00
150.00	7.42	5.08	0.00
155.00	7.42	5.08	0.00
160.00	7.42	5.08	0.00
165.00	7.42	5.08	0.00
170.00	7.42	5.08	0.00
175.00	7.42	5.08	0.00
180.00	7.42	5.08	0.00
185.00	7.42	5.08	0.00
190.00	7.42	5.08	0.00
195.00	7.42	5.08	0.00
200.00	7.42	5.08	0.00
205.00	7.42	5.08	0.00
210.00	7.42	5.08	0.00
215.00	7.42	5.08	0.00
220.00	7.42	5.08	0.00
225.00	7.42	5.08	0.00
230.00	7.42	5.08	0.00
235.00	7.42	5.08	0.00
240.00	7.42	5.08	0.00

PROPOSED CONDITIONS



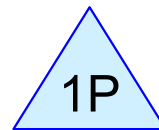
DRAINS NORTH TO
AMIRA APTS SITE -
UNTREATED



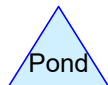
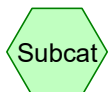
DRAINS SOUTH TO
EX. STORM SEWER -
UNTREATED



DRAINS TO POND 1
THEN SOUTH TO EX.
STORM SEWER



UNDERGROUND
POND 1



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Printed 6/5/2024

Page 2

Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1.1" RAINFALL	MSE 24-hr	3	Default	24.00	1	1.10	2
2	1.5" RAINFALL	MSE 24-hr	3	Default	24.00	1	1.50	2
3	1y 24hr AT-14	MSE 24-hr	3	Default	24.00	1	2.50	2
4	2y 24hr AT-14	MSE 24-hr	3	Default	24.00	1	2.88	2
5	10y 24hr AT-14	MSE 24-hr	3	Default	24.00	1	4.29	2
6	100y 24hr AT-14	MSE 24-hr	3	Default	24.00	1	7.42	2

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Printed 6/5/2024

Page 3

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.752	80	>75% Grass cover, Good, HSG D (PR1A, PR1B, PR2)
1.568	98	Paved parking, HSG D (PR1A, PR1B, PR2)
2.320	92	TOTAL AREA

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Printed 6/5/2024

Page 4

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
2.320	HSG D	PR1A, PR1B, PR2
0.000	Other	
2.320		TOTAL AREA

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Printed 6/5/2024

Page 5

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	0.752	0.000	0.752	>75% Grass cover, Good	PR1A, PR1B, PR2
0.000	0.000	0.000	1.568	0.000	1.568	Paved parking	PR1A, PR1B, PR2
0.000	0.000	0.000	2.320	0.000	2.320	TOTAL AREA	

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Printed 6/5/2024

Page 6

Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)	Node Name
1	1P	961.90	961.38	103.0	0.0050	0.012	0.0	15.0	0.0	

24132 PROPOSED

MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 7

Time span=0.00-240.00 hrs, dt=0.01 hrs, 24001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

SubcatchmentPR1A: DRAINS TO POND 1 Runoff Area=73,474 sf 83.48% Impervious Runoff Depth=0.76"
Tc=6.0 min CN=WQ Runoff=2.18 cfs 0.107 af

SubcatchmentPR1B: DRAINS SOUTH TO Runoff Area=14,493 sf 28.85% Impervious Runoff Depth=0.34"
Tc=6.0 min CN=WQ Runoff=0.17 cfs 0.009 af

SubcatchmentPR2: DRAINS NORTH TO Runoff Area=13,090 sf 21.37% Impervious Runoff Depth=0.28"
Tc=6.0 min CN=WQ Runoff=0.13 cfs 0.007 af

Pond 1P: UNDERGROUNDPOND 1 Peak Elev=966.05' Storage=9,627 cf Inflow=2.18 cfs 0.107 af
Outflow=0.44 cfs 0.107 af

Total Runoff Area = 2.320 ac Runoff Volume = 0.123 af Average Runoff Depth = 0.64"
32.40% Pervious = 0.752 ac 67.60% Impervious = 1.568 ac

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Printed 6/5/2024

Page 8

Summary for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Runoff = 2.18 cfs @ 12.13 hrs, Volume= 0.107 af, Depth= 0.76"
Routed to Pond 1P : UNDERGROUND POND 1

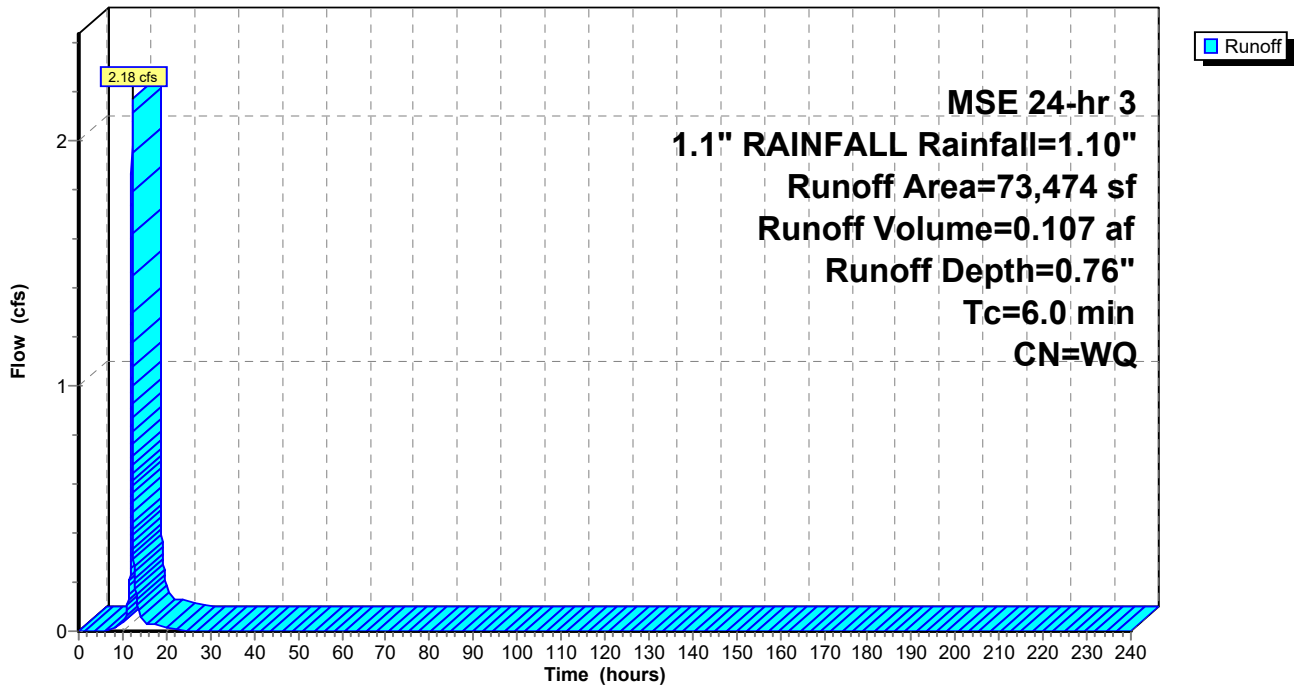
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Area (sf)	CN	Description
61,334	98	Paved parking, HSG D
12,140	80	>75% Grass cover, Good, HSG D
73,474		Weighted Average
12,140		16.52% Pervious Area
61,334		83.48% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Printed 6/5/2024

Page 9

Hydrograph for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.04	0.00	0.00
10.00	0.15	0.00	0.03
15.00	0.99	0.55	0.05
20.00	1.07	0.63	0.02
25.00	1.10	0.65	0.00
30.00	1.10	0.65	0.00
35.00	1.10	0.65	0.00
40.00	1.10	0.65	0.00
45.00	1.10	0.65	0.00
50.00	1.10	0.65	0.00
55.00	1.10	0.65	0.00
60.00	1.10	0.65	0.00
65.00	1.10	0.65	0.00
70.00	1.10	0.65	0.00
75.00	1.10	0.65	0.00
80.00	1.10	0.65	0.00
85.00	1.10	0.65	0.00
90.00	1.10	0.65	0.00
95.00	1.10	0.65	0.00
100.00	1.10	0.65	0.00
105.00	1.10	0.65	0.00
110.00	1.10	0.65	0.00
115.00	1.10	0.65	0.00
120.00	1.10	0.65	0.00
125.00	1.10	0.65	0.00
130.00	1.10	0.65	0.00
135.00	1.10	0.65	0.00
140.00	1.10	0.65	0.00
145.00	1.10	0.65	0.00
150.00	1.10	0.65	0.00
155.00	1.10	0.65	0.00
160.00	1.10	0.65	0.00
165.00	1.10	0.65	0.00
170.00	1.10	0.65	0.00
175.00	1.10	0.65	0.00
180.00	1.10	0.65	0.00
185.00	1.10	0.65	0.00
190.00	1.10	0.65	0.00
195.00	1.10	0.65	0.00
200.00	1.10	0.65	0.00
205.00	1.10	0.65	0.00
210.00	1.10	0.65	0.00
215.00	1.10	0.65	0.00
220.00	1.10	0.65	0.00
225.00	1.10	0.65	0.00
230.00	1.10	0.65	0.00
235.00	1.10	0.65	0.00
240.00	1.10	0.65	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Printed 6/5/2024

Page 10

Summary for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Runoff = 0.17 cfs @ 12.14 hrs, Volume= 0.009 af, Depth= 0.34"

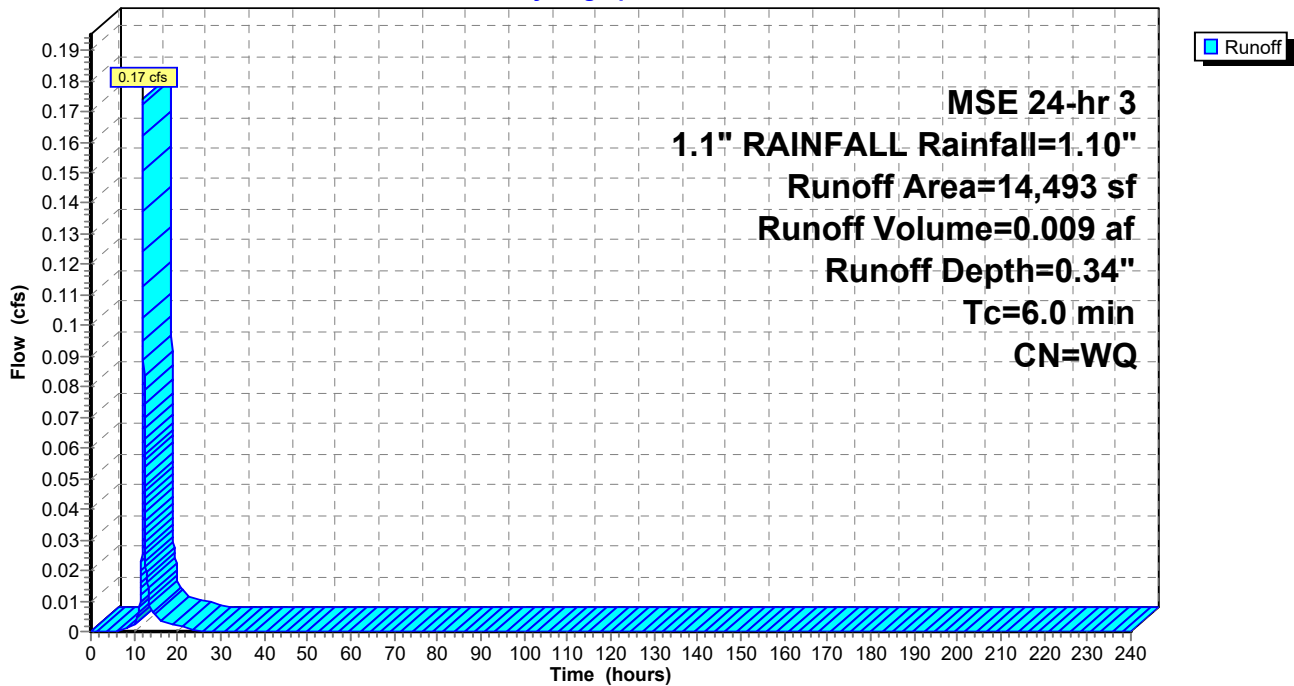
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Area (sf)	CN	Description
4,181	98	Paved parking, HSG D
10,312	80	>75% Grass cover, Good, HSG D
14,493		Weighted Average
10,312		71.15% Pervious Area
4,181		28.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Printed 6/5/2024

Page 11

Hydrograph for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.04	0.00	0.00
10.00	0.15	0.00	0.00
15.00	0.99	0.17	0.01
20.00	1.07	0.21	0.00
25.00	1.10	0.22	0.00
30.00	1.10	0.22	0.00
35.00	1.10	0.22	0.00
40.00	1.10	0.22	0.00
45.00	1.10	0.22	0.00
50.00	1.10	0.22	0.00
55.00	1.10	0.22	0.00
60.00	1.10	0.22	0.00
65.00	1.10	0.22	0.00
70.00	1.10	0.22	0.00
75.00	1.10	0.22	0.00
80.00	1.10	0.22	0.00
85.00	1.10	0.22	0.00
90.00	1.10	0.22	0.00
95.00	1.10	0.22	0.00
100.00	1.10	0.22	0.00
105.00	1.10	0.22	0.00
110.00	1.10	0.22	0.00
115.00	1.10	0.22	0.00
120.00	1.10	0.22	0.00
125.00	1.10	0.22	0.00
130.00	1.10	0.22	0.00
135.00	1.10	0.22	0.00
140.00	1.10	0.22	0.00
145.00	1.10	0.22	0.00
150.00	1.10	0.22	0.00
155.00	1.10	0.22	0.00
160.00	1.10	0.22	0.00
165.00	1.10	0.22	0.00
170.00	1.10	0.22	0.00
175.00	1.10	0.22	0.00
180.00	1.10	0.22	0.00
185.00	1.10	0.22	0.00
190.00	1.10	0.22	0.00
195.00	1.10	0.22	0.00
200.00	1.10	0.22	0.00
205.00	1.10	0.22	0.00
210.00	1.10	0.22	0.00
215.00	1.10	0.22	0.00
220.00	1.10	0.22	0.00
225.00	1.10	0.22	0.00
230.00	1.10	0.22	0.00
235.00	1.10	0.22	0.00
240.00	1.10	0.22	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Printed 6/5/2024

Page 12

Summary for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Runoff = 0.13 cfs @ 12.14 hrs, Volume= 0.007 af, Depth= 0.28"

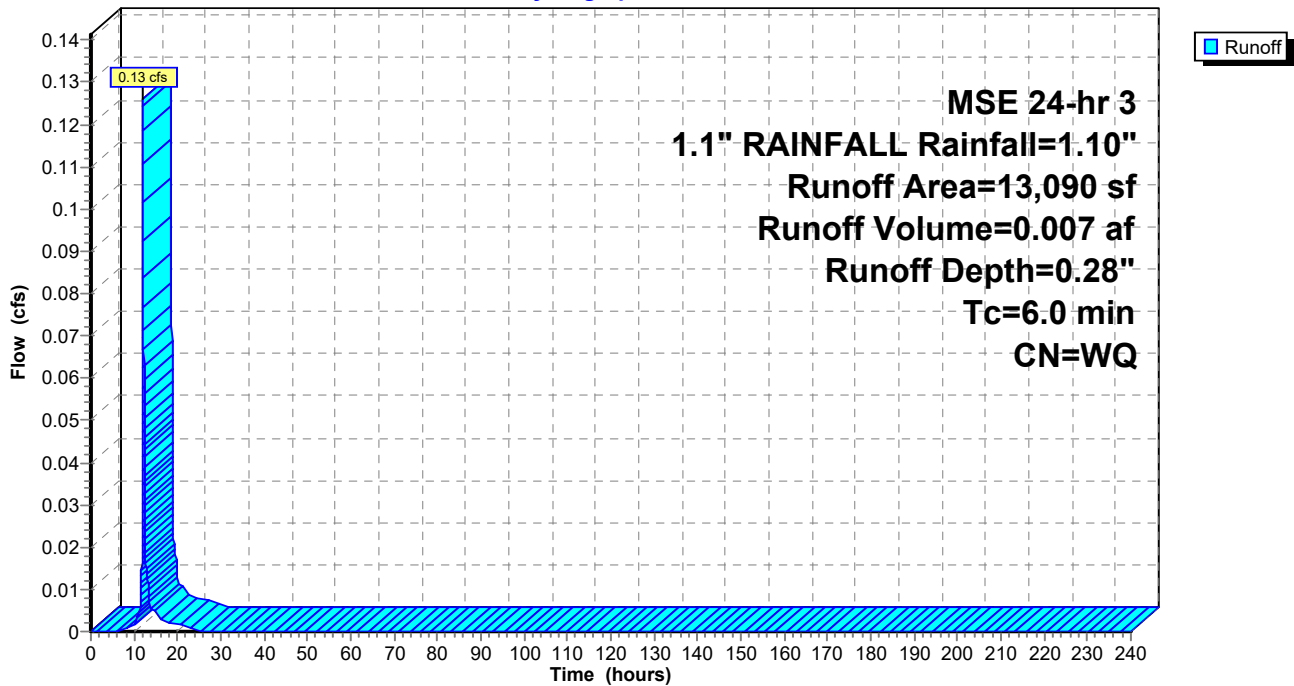
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Area (sf)	CN	Description
2,797	98	Paved parking, HSG D
10,293	80	>75% Grass cover, Good, HSG D
13,090		Weighted Average
10,293		78.63% Pervious Area
2,797		21.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Printed 6/5/2024

Page 13

Hydrograph for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.04	0.00	0.00
10.00	0.15	0.00	0.00
15.00	0.99	0.15	0.00
20.00	1.07	0.18	0.00
25.00	1.10	0.20	0.00
30.00	1.10	0.20	0.00
35.00	1.10	0.20	0.00
40.00	1.10	0.20	0.00
45.00	1.10	0.20	0.00
50.00	1.10	0.20	0.00
55.00	1.10	0.20	0.00
60.00	1.10	0.20	0.00
65.00	1.10	0.20	0.00
70.00	1.10	0.20	0.00
75.00	1.10	0.20	0.00
80.00	1.10	0.20	0.00
85.00	1.10	0.20	0.00
90.00	1.10	0.20	0.00
95.00	1.10	0.20	0.00
100.00	1.10	0.20	0.00
105.00	1.10	0.20	0.00
110.00	1.10	0.20	0.00
115.00	1.10	0.20	0.00
120.00	1.10	0.20	0.00
125.00	1.10	0.20	0.00
130.00	1.10	0.20	0.00
135.00	1.10	0.20	0.00
140.00	1.10	0.20	0.00
145.00	1.10	0.20	0.00
150.00	1.10	0.20	0.00
155.00	1.10	0.20	0.00
160.00	1.10	0.20	0.00
165.00	1.10	0.20	0.00
170.00	1.10	0.20	0.00
175.00	1.10	0.20	0.00
180.00	1.10	0.20	0.00
185.00	1.10	0.20	0.00
190.00	1.10	0.20	0.00
195.00	1.10	0.20	0.00
200.00	1.10	0.20	0.00
205.00	1.10	0.20	0.00
210.00	1.10	0.20	0.00
215.00	1.10	0.20	0.00
220.00	1.10	0.20	0.00
225.00	1.10	0.20	0.00
230.00	1.10	0.20	0.00
235.00	1.10	0.20	0.00
240.00	1.10	0.20	0.00

24132 PROPOSED

MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 14

Summary for Pond 1P: UNDERGROUND POND 1

Inflow Area = 1.687 ac, 83.48% Impervious, Inflow Depth = 0.76" for 1.1" RAINFALL event
 Inflow = 2.18 cfs @ 12.13 hrs, Volume= 0.107 af
 Outflow = 0.44 cfs @ 12.40 hrs, Volume= 0.107 af, Atten= 80%, Lag= 16.2 min
 Primary = 0.44 cfs @ 12.40 hrs, Volume= 0.107 af

Routing by Stor-Ind method, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 Starting Elev= 965.40' Surf.Area= 4,484 sf Storage= 7,467 cf
 Peak Elev= 966.05' @ 12.40 hrs Surf.Area= 4,484 sf Storage= 9,627 cf (2,159 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 132.9 min (908.9 - 776.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	962.40'	0 cf	19.00'W x 236.00'L x 7.50'H Field A 33,630 cf Overall - 18,242 cf Embedded = 15,388 cf x 0.0% Voids
#2A	962.40'	18,242 cf	CMP Round 84 x 22 Inside #1 Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf Overall Size= 84.0"W x 84.0"H x 20.00'L 22 Chambers in 2 Rows 17.00' Header x 38.48 sf x 2 = 1,308.5 cf Inside
		18,242 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	961.90'	15.0" Round Culvert L= 103.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 961.90' / 961.38' S= 0.0050 '/' Cc= 0.900 n= 0.012 Concrete pipe, finished, Flow Area= 1.23 sf
#2	Device 1	965.40'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	966.40'	16.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.44 cfs @ 12.40 hrs HW=966.05' (Free Discharge)

- ↑ **1=Culvert** (Passes 0.44 cfs of 9.41 cfs potential flow)
- ↑ **2=Orifice/Grate** (Orifice Controls 0.44 cfs @ 3.21 fps)
- ↑ **3=Orifice/Grate** (Controls 0.00 cfs)

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Printed 6/5/2024

Page 15

Pond 1P: UNDERGROUND POND 1 - Chamber Wizard Field A

Chamber Model = CMP Round 84 (Round Corrugated Metal Pipe)

Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf

Overall Size= 84.0"W x 84.0"H x 20.00'L

84.0" Wide + 36.0" Spacing = 120.0" C-C Row Spacing

11 Chambers/Row x 20.00' Long +7.00' Header x 2 = 234.00' Row Length +12.0" End Stone x 2 = 236.00' Base Length

2 Rows x 84.0" Wide + 36.0" Spacing x 1 + 12.0" Side Stone x 2 = 19.00' Base Width

84.0" Chamber Height + 6.0" Stone Cover = 7.50' Field Height

22 Chambers x 769.7 cf + 17.00' Header x 38.48 sf x 2 = 18,241.7 cf Chamber Storage

33,630.0 cf Field - 18,241.7 cf Chambers = 15,388.3 cf Stone x 0.0% Voids = 0.0 cf Stone Storage

Chamber Storage = 18,241.7 cf = 0.419 af

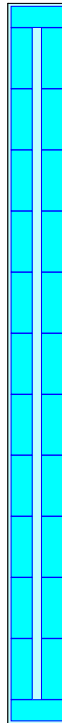
Overall Storage Efficiency = 54.2%

Overall System Size = 236.00' x 19.00' x 7.50'

22 Chambers

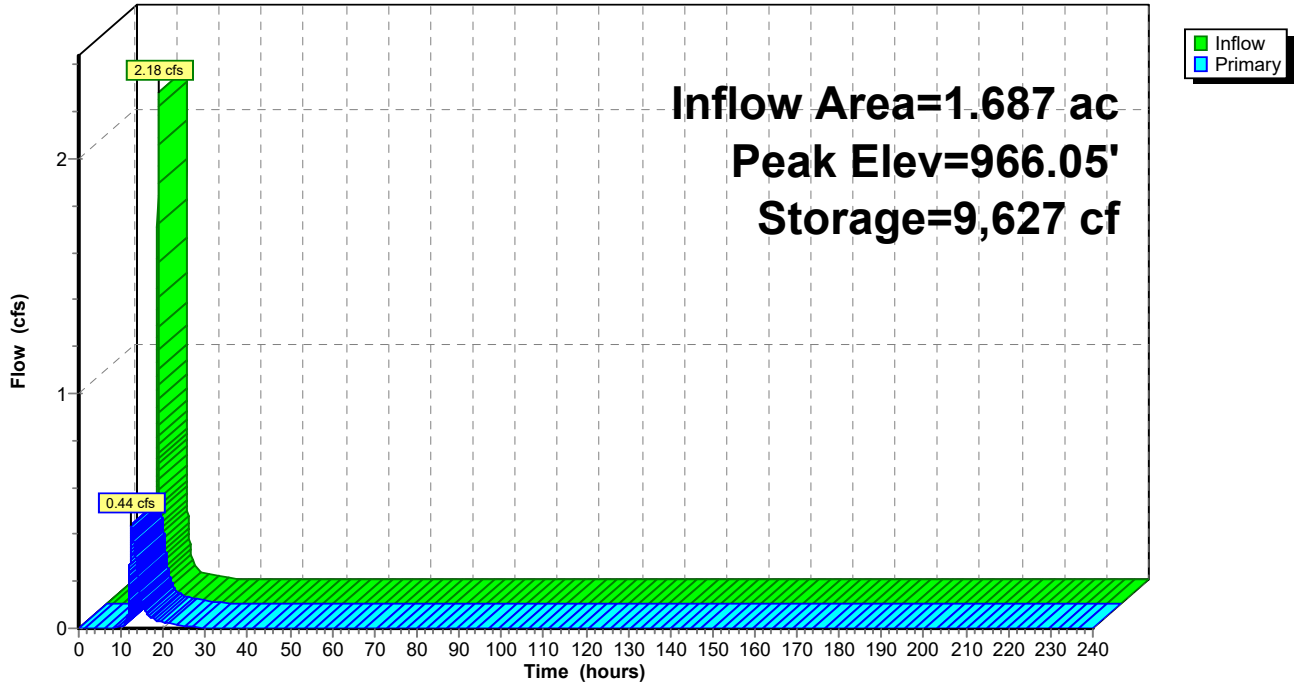
1,245.6 cy Field

569.9 cy Stone



Pond 1P: UNDERGROUND POND 1

Hydrograph



24132 PROPOSED

MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 17

Hydrograph for Pond 1P: UNDERGROUND POND 1

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
0.00	0.00	7,467	965.40	0.00
5.00	0.00	7,467	965.40	0.00
10.00	0.03	7,621	965.45	0.01
15.00	0.05	8,165	965.61	0.11
20.00	0.02	7,769	965.49	0.02
25.00	0.00	7,635	965.45	0.01
30.00	0.00	7,535	965.42	0.00
35.00	0.00	7,507	965.41	0.00
40.00	0.00	7,499	965.41	0.00
45.00	0.00	7,494	965.41	0.00
50.00	0.00	7,490	965.41	0.00
55.00	0.00	7,487	965.41	0.00
60.00	0.00	7,484	965.41	0.00
65.00	0.00	7,482	965.40	0.00
70.00	0.00	7,480	965.40	0.00
75.00	0.00	7,478	965.40	0.00
80.00	0.00	7,476	965.40	0.00
85.00	0.00	7,475	965.40	0.00
90.00	0.00	7,474	965.40	0.00
95.00	0.00	7,473	965.40	0.00
100.00	0.00	7,472	965.40	0.00
105.00	0.00	7,471	965.40	0.00
110.00	0.00	7,471	965.40	0.00
115.00	0.00	7,470	965.40	0.00
120.00	0.00	7,470	965.40	0.00
125.00	0.00	7,470	965.40	0.00
130.00	0.00	7,469	965.40	0.00
135.00	0.00	7,469	965.40	0.00
140.00	0.00	7,469	965.40	0.00
145.00	0.00	7,469	965.40	0.00
150.00	0.00	7,468	965.40	0.00
155.00	0.00	7,468	965.40	0.00
160.00	0.00	7,468	965.40	0.00
165.00	0.00	7,468	965.40	0.00
170.00	0.00	7,468	965.40	0.00
175.00	0.00	7,468	965.40	0.00
180.00	0.00	7,468	965.40	0.00
185.00	0.00	7,468	965.40	0.00
190.00	0.00	7,468	965.40	0.00
195.00	0.00	7,468	965.40	0.00
200.00	0.00	7,468	965.40	0.00
205.00	0.00	7,468	965.40	0.00
210.00	0.00	7,468	965.40	0.00
215.00	0.00	7,468	965.40	0.00
220.00	0.00	7,468	965.40	0.00
225.00	0.00	7,468	965.40	0.00
230.00	0.00	7,468	965.40	0.00
235.00	0.00	7,468	965.40	0.00
240.00	0.00	7,468	965.40	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.1" RAINFALL Rainfall=1.10"

Printed 6/5/2024

Page 18

Stage-Area-Storage for Pond 1P: UNDERGROUND POND 1

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
962.40	0	965.00	6,168	967.60	14,531
962.45	12	965.05	6,328	967.65	14,675
962.50	52	965.10	6,490	967.70	14,818
962.55	96	965.15	6,652	967.75	14,960
962.60	148	965.20	6,814	967.80	15,100
962.65	207	965.25	6,977	967.85	15,239
962.70	271	965.30	7,140	967.90	15,376
962.75	341	965.35	7,303	967.95	15,511
962.80	416	965.40	7,467	968.00	15,644
962.85	495	965.45	7,632	968.05	15,776
962.90	578	965.50	7,797	968.10	15,906
962.95	666	965.55	7,961	968.15	16,034
963.00	757	965.60	8,127	968.20	16,160
963.05	851	965.65	8,292	968.25	16,284
963.10	949	965.70	8,458	968.30	16,406
963.15	1,050	965.75	8,623	968.35	16,526
963.20	1,155	965.80	8,789	968.40	16,643
963.25	1,262	965.85	8,955	968.45	16,758
963.30	1,371	965.90	9,121	968.50	16,870
963.35	1,484	965.95	9,287	968.55	16,980
963.40	1,599	966.00	9,453	968.60	17,087
963.45	1,716	966.05	9,618	968.65	17,191
963.50	1,835	966.10	9,784	968.70	17,292
963.55	1,957	966.15	9,950	968.75	17,390
963.60	2,081	966.20	10,115	968.80	17,485
963.65	2,207	966.25	10,280	968.85	17,576
963.70	2,335	966.30	10,445	968.90	17,663
963.75	2,465	966.35	10,610	968.95	17,747
963.80	2,597	966.40	10,774	969.00	17,826
963.85	2,731	966.45	10,938	969.05	17,901
963.90	2,866	966.50	11,102	969.10	17,970
963.95	3,003	966.55	11,265	969.15	18,035
964.00	3,142	966.60	11,428	969.20	18,093
964.05	3,282	966.65	11,590	969.25	18,145
964.10	3,423	966.70	11,752	969.30	18,189
964.15	3,566	966.75	11,913	969.35	18,222
964.20	3,711	966.80	12,074	969.40	18,242
964.25	3,856	966.85	12,234	969.45	18,242
964.30	4,003	966.90	12,393	969.50	18,242
964.35	4,151	966.95	12,552	969.55	18,242
964.40	4,301	967.00	12,710	969.60	18,242
964.45	4,451	967.05	12,867	969.65	18,242
964.50	4,603	967.10	13,023	969.70	18,242
964.55	4,755	967.15	13,178	969.75	18,242
964.60	4,909	967.20	13,333	969.80	18,242
964.65	5,063	967.25	13,486	969.85	18,242
964.70	5,219	967.30	13,639	969.90	18,242
964.75	5,375	967.35	13,791		
964.80	5,532	967.40	13,941		
964.85	5,690	967.45	14,090		
964.90	5,849	967.50	14,238		
964.95	6,008	967.55	14,385		

24132 PROPOSED

MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 19

Time span=0.00-240.00 hrs, dt=0.01 hrs, 24001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

SubcatchmentPR1A: DRAINS TO POND 1 Runoff Area=73,474 sf 83.48% Impervious Runoff Depth=1.12"
Tc=6.0 min CN=WQ Runoff=3.15 cfs 0.157 af

SubcatchmentPR1B: DRAINS SOUTH TO Runoff Area=14,493 sf 28.85% Impervious Runoff Depth=0.57"
Tc=6.0 min CN=WQ Runoff=0.32 cfs 0.016 af

SubcatchmentPR2: DRAINS NORTH TO Runoff Area=13,090 sf 21.37% Impervious Runoff Depth=0.50"
Tc=6.0 min CN=WQ Runoff=0.25 cfs 0.012 af

Pond 1P: UNDERGROUND POND 1 Peak Elev=966.35' Storage=10,619 cf Inflow=3.15 cfs 0.157 af
Outflow=0.57 cfs 0.157 af

Total Runoff Area = 2.320 ac Runoff Volume = 0.185 af Average Runoff Depth = 0.96"
32.40% Pervious = 0.752 ac 67.60% Impervious = 1.568 ac

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Printed 6/5/2024

Page 20

Summary for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Runoff = 3.15 cfs @ 12.13 hrs, Volume= 0.157 af, Depth= 1.12"
Routed to Pond 1P : UNDERGROUND POND 1

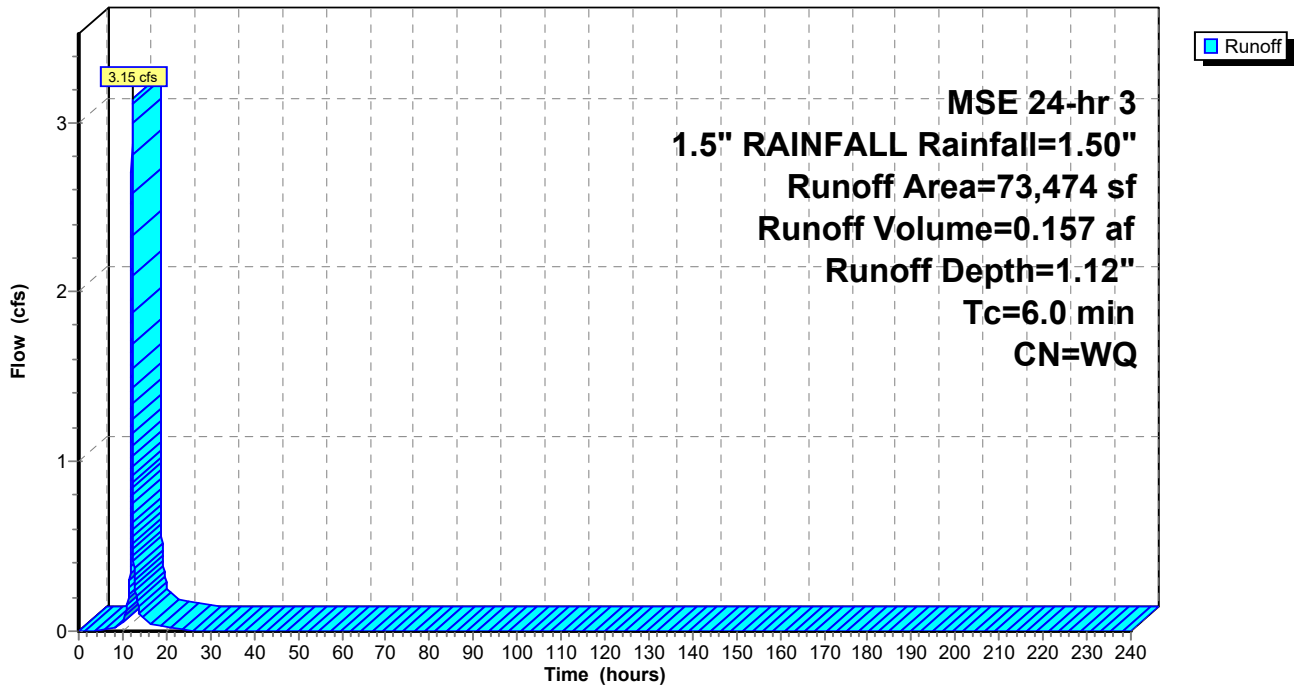
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Area (sf)	CN	Description
61,334	98	Paved parking, HSG D
12,140	80	>75% Grass cover, Good, HSG D
73,474		Weighted Average
12,140		16.52% Pervious Area
61,334		83.48% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Printed 6/5/2024

Page 21

Hydrograph for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.06	0.00	0.00
10.00	0.21	0.02	0.05
15.00	1.35	0.87	0.07
20.00	1.46	0.98	0.02
25.00	1.50	1.01	0.00
30.00	1.50	1.01	0.00
35.00	1.50	1.01	0.00
40.00	1.50	1.01	0.00
45.00	1.50	1.01	0.00
50.00	1.50	1.01	0.00
55.00	1.50	1.01	0.00
60.00	1.50	1.01	0.00
65.00	1.50	1.01	0.00
70.00	1.50	1.01	0.00
75.00	1.50	1.01	0.00
80.00	1.50	1.01	0.00
85.00	1.50	1.01	0.00
90.00	1.50	1.01	0.00
95.00	1.50	1.01	0.00
100.00	1.50	1.01	0.00
105.00	1.50	1.01	0.00
110.00	1.50	1.01	0.00
115.00	1.50	1.01	0.00
120.00	1.50	1.01	0.00
125.00	1.50	1.01	0.00
130.00	1.50	1.01	0.00
135.00	1.50	1.01	0.00
140.00	1.50	1.01	0.00
145.00	1.50	1.01	0.00
150.00	1.50	1.01	0.00
155.00	1.50	1.01	0.00
160.00	1.50	1.01	0.00
165.00	1.50	1.01	0.00
170.00	1.50	1.01	0.00
175.00	1.50	1.01	0.00
180.00	1.50	1.01	0.00
185.00	1.50	1.01	0.00
190.00	1.50	1.01	0.00
195.00	1.50	1.01	0.00
200.00	1.50	1.01	0.00
205.00	1.50	1.01	0.00
210.00	1.50	1.01	0.00
215.00	1.50	1.01	0.00
220.00	1.50	1.01	0.00
225.00	1.50	1.01	0.00
230.00	1.50	1.01	0.00
235.00	1.50	1.01	0.00
240.00	1.50	1.01	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Printed 6/5/2024

Page 22

Summary for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Runoff = 0.32 cfs @ 12.14 hrs, Volume= 0.016 af, Depth= 0.57"

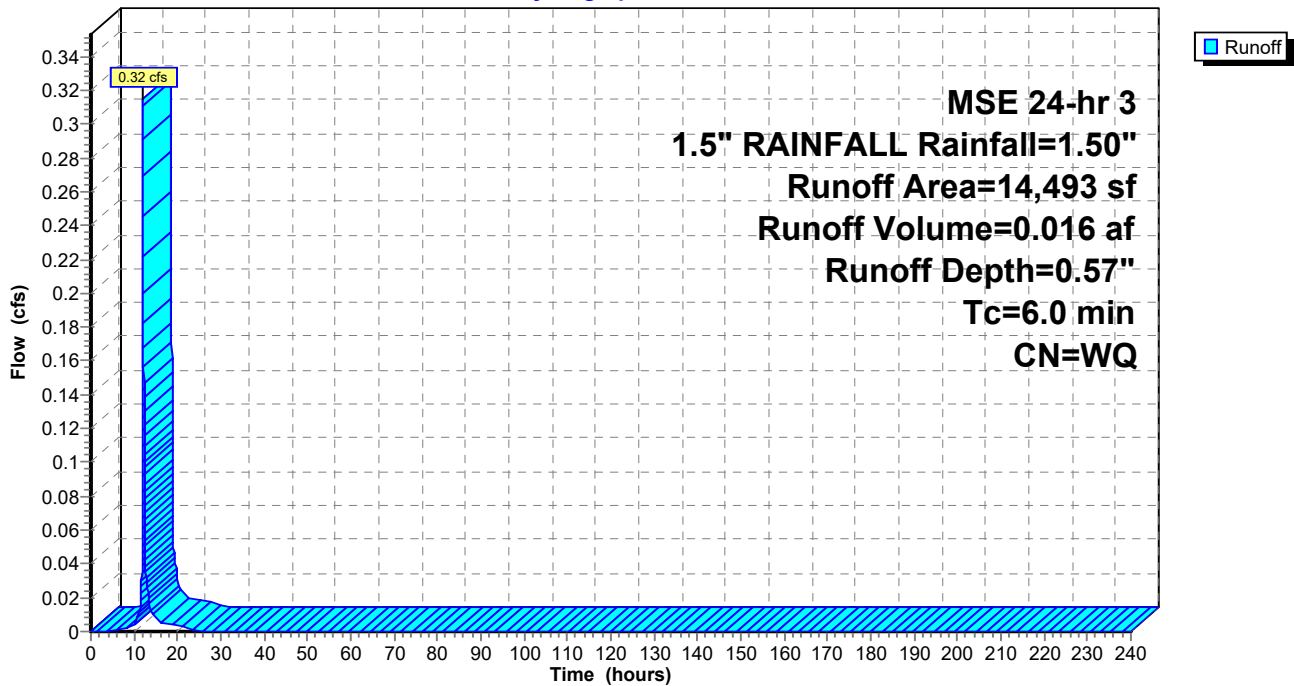
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Area (sf)	CN	Description
4,181	98	Paved parking, HSG D
10,312	80	>75% Grass cover, Good, HSG D
14,493		Weighted Average
10,312		71.15% Pervious Area
4,181		28.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Printed 6/5/2024

Page 23

Hydrograph for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.06	0.00	0.00
10.00	0.21	0.00	0.00
15.00	1.35	0.36	0.01
20.00	1.46	0.43	0.00
25.00	1.50	0.45	0.00
30.00	1.50	0.45	0.00
35.00	1.50	0.45	0.00
40.00	1.50	0.45	0.00
45.00	1.50	0.45	0.00
50.00	1.50	0.45	0.00
55.00	1.50	0.45	0.00
60.00	1.50	0.45	0.00
65.00	1.50	0.45	0.00
70.00	1.50	0.45	0.00
75.00	1.50	0.45	0.00
80.00	1.50	0.45	0.00
85.00	1.50	0.45	0.00
90.00	1.50	0.45	0.00
95.00	1.50	0.45	0.00
100.00	1.50	0.45	0.00
105.00	1.50	0.45	0.00
110.00	1.50	0.45	0.00
115.00	1.50	0.45	0.00
120.00	1.50	0.45	0.00
125.00	1.50	0.45	0.00
130.00	1.50	0.45	0.00
135.00	1.50	0.45	0.00
140.00	1.50	0.45	0.00
145.00	1.50	0.45	0.00
150.00	1.50	0.45	0.00
155.00	1.50	0.45	0.00
160.00	1.50	0.45	0.00
165.00	1.50	0.45	0.00
170.00	1.50	0.45	0.00
175.00	1.50	0.45	0.00
180.00	1.50	0.45	0.00
185.00	1.50	0.45	0.00
190.00	1.50	0.45	0.00
195.00	1.50	0.45	0.00
200.00	1.50	0.45	0.00
205.00	1.50	0.45	0.00
210.00	1.50	0.45	0.00
215.00	1.50	0.45	0.00
220.00	1.50	0.45	0.00
225.00	1.50	0.45	0.00
230.00	1.50	0.45	0.00
235.00	1.50	0.45	0.00
240.00	1.50	0.45	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Printed 6/5/2024

Page 24

Summary for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Runoff = 0.25 cfs @ 12.14 hrs, Volume= 0.012 af, Depth= 0.50"

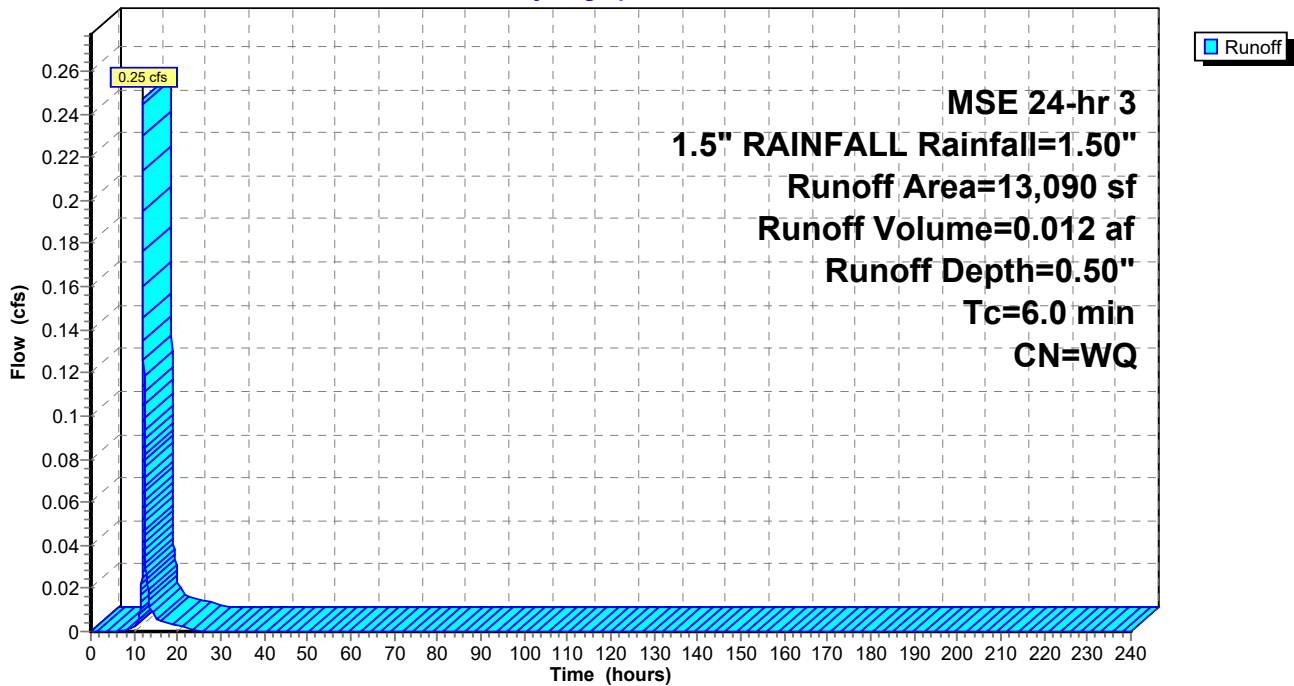
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Area (sf)	CN	Description
2,797	98	Paved parking, HSG D
10,293	80	>75% Grass cover, Good, HSG D
13,090		Weighted Average
10,293		78.63% Pervious Area
2,797		21.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Printed 6/5/2024

Page 25

Hydrograph for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.06	0.00	0.00
10.00	0.21	0.00	0.00
15.00	1.35	0.32	0.01
20.00	1.46	0.39	0.00
25.00	1.50	0.41	0.00
30.00	1.50	0.41	0.00
35.00	1.50	0.41	0.00
40.00	1.50	0.41	0.00
45.00	1.50	0.41	0.00
50.00	1.50	0.41	0.00
55.00	1.50	0.41	0.00
60.00	1.50	0.41	0.00
65.00	1.50	0.41	0.00
70.00	1.50	0.41	0.00
75.00	1.50	0.41	0.00
80.00	1.50	0.41	0.00
85.00	1.50	0.41	0.00
90.00	1.50	0.41	0.00
95.00	1.50	0.41	0.00
100.00	1.50	0.41	0.00
105.00	1.50	0.41	0.00
110.00	1.50	0.41	0.00
115.00	1.50	0.41	0.00
120.00	1.50	0.41	0.00
125.00	1.50	0.41	0.00
130.00	1.50	0.41	0.00
135.00	1.50	0.41	0.00
140.00	1.50	0.41	0.00
145.00	1.50	0.41	0.00
150.00	1.50	0.41	0.00
155.00	1.50	0.41	0.00
160.00	1.50	0.41	0.00
165.00	1.50	0.41	0.00
170.00	1.50	0.41	0.00
175.00	1.50	0.41	0.00
180.00	1.50	0.41	0.00
185.00	1.50	0.41	0.00
190.00	1.50	0.41	0.00
195.00	1.50	0.41	0.00
200.00	1.50	0.41	0.00
205.00	1.50	0.41	0.00
210.00	1.50	0.41	0.00
215.00	1.50	0.41	0.00
220.00	1.50	0.41	0.00
225.00	1.50	0.41	0.00
230.00	1.50	0.41	0.00
235.00	1.50	0.41	0.00
240.00	1.50	0.41	0.00

24132 PROPOSED

MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 26

Summary for Pond 1P: UNDERGROUND POND 1

Inflow Area = 1.687 ac, 83.48% Impervious, Inflow Depth = 1.12" for 1.5" RAINFALL event
 Inflow = 3.15 cfs @ 12.13 hrs, Volume= 0.157 af
 Outflow = 0.57 cfs @ 12.44 hrs, Volume= 0.157 af, Atten= 82%, Lag= 18.3 min
 Primary = 0.57 cfs @ 12.44 hrs, Volume= 0.157 af

Routing by Stor-Ind method, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 Starting Elev= 965.40' Surf.Area= 4,484 sf Storage= 7,467 cf
 Peak Elev= 966.35' @ 12.44 hrs Surf.Area= 4,484 sf Storage= 10,619 cf (3,152 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 121.0 min (890.9 - 769.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	962.40'	0 cf	19.00'W x 236.00'L x 7.50'H Field A 33,630 cf Overall - 18,242 cf Embedded = 15,388 cf x 0.0% Voids
#2A	962.40'	18,242 cf	CMP Round 84 x 22 Inside #1 Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf Overall Size= 84.0"W x 84.0"H x 20.00'L 22 Chambers in 2 Rows 17.00' Header x 38.48 sf x 2 = 1,308.5 cf Inside
		18,242 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	961.90'	15.0" Round Culvert L= 103.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 961.90' / 961.38' S= 0.0050 '/' Cc= 0.900 n= 0.012 Concrete pipe, finished, Flow Area= 1.23 sf
#2	Device 1	965.40'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	966.40'	16.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.57 cfs @ 12.44 hrs HW=966.35' (Free Discharge)

- ↑ **1=Culvert** (Passes 0.57 cfs of 9.82 cfs potential flow)
- ↑ **2=Orifice/Grate** (Orifice Controls 0.57 cfs @ 4.15 fps)
- ↑ **3=Orifice/Grate** (Controls 0.00 cfs)

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Printed 6/5/2024

Page 27

Pond 1P: UNDERGROUND POND 1 - Chamber Wizard Field A

Chamber Model = CMP Round 84 (Round Corrugated Metal Pipe)

Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf

Overall Size= 84.0"W x 84.0"H x 20.00'L

84.0" Wide + 36.0" Spacing = 120.0" C-C Row Spacing

11 Chambers/Row x 20.00' Long +7.00' Header x 2 = 234.00' Row Length +12.0" End Stone x 2 = 236.00'

Base Length

2 Rows x 84.0" Wide + 36.0" Spacing x 1 + 12.0" Side Stone x 2 = 19.00' Base Width

84.0" Chamber Height + 6.0" Stone Cover = 7.50' Field Height

22 Chambers x 769.7 cf + 17.00' Header x 38.48 sf x 2 = 18,241.7 cf Chamber Storage

33,630.0 cf Field - 18,241.7 cf Chambers = 15,388.3 cf Stone x 0.0% Voids = 0.0 cf Stone Storage

Chamber Storage = 18,241.7 cf = 0.419 af

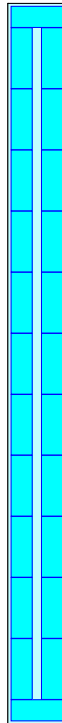
Overall Storage Efficiency = 54.2%

Overall System Size = 236.00' x 19.00' x 7.50'

22 Chambers

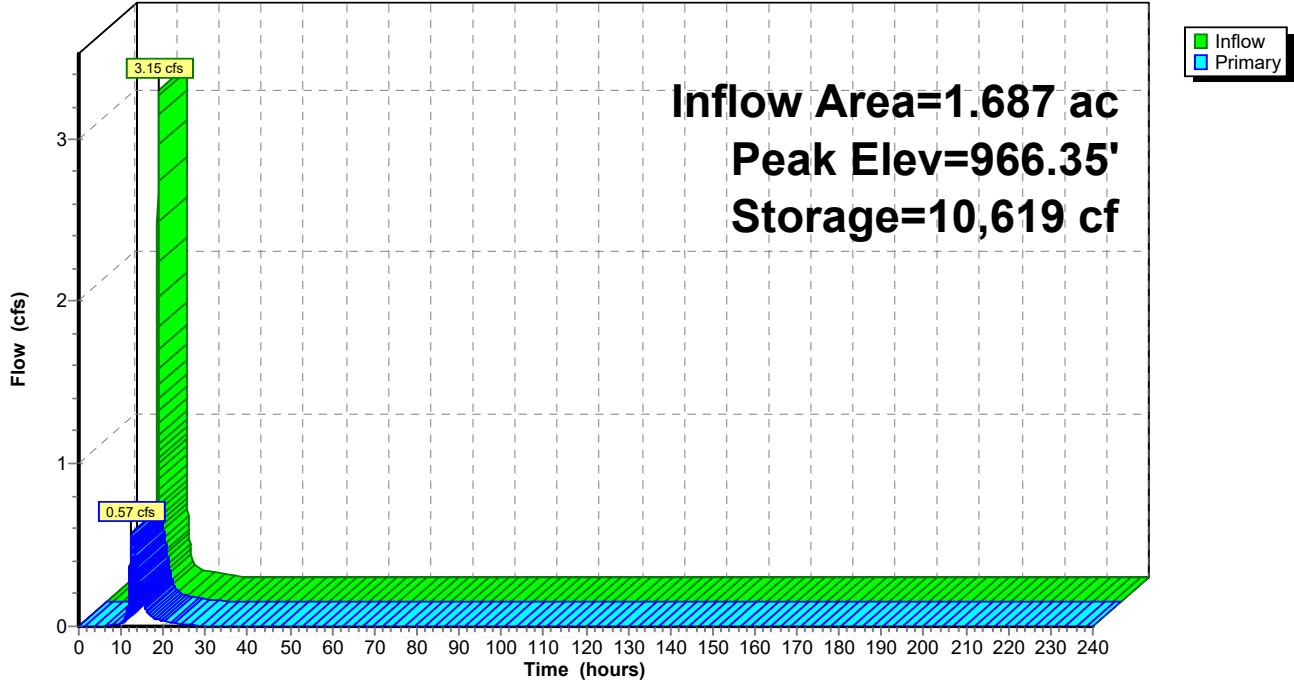
1,245.6 cy Field

569.9 cy Stone



Pond 1P: UNDERGROUND POND 1

Hydrograph



24132 PROPOSED

MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 29

Hydrograph for Pond 1P: UNDERGROUND POND 1

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
0.00	0.00	7,467	965.40	0.00
5.00	0.00	7,472	965.40	0.00
10.00	0.05	7,738	965.48	0.02
15.00	0.07	8,422	965.69	0.19
20.00	0.02	7,823	965.51	0.03
25.00	0.00	7,661	965.46	0.01
30.00	0.00	7,542	965.42	0.00
35.00	0.00	7,509	965.41	0.00
40.00	0.00	7,500	965.41	0.00
45.00	0.00	7,495	965.41	0.00
50.00	0.00	7,491	965.41	0.00
55.00	0.00	7,487	965.41	0.00
60.00	0.00	7,485	965.41	0.00
65.00	0.00	7,482	965.40	0.00
70.00	0.00	7,480	965.40	0.00
75.00	0.00	7,478	965.40	0.00
80.00	0.00	7,476	965.40	0.00
85.00	0.00	7,475	965.40	0.00
90.00	0.00	7,474	965.40	0.00
95.00	0.00	7,473	965.40	0.00
100.00	0.00	7,472	965.40	0.00
105.00	0.00	7,472	965.40	0.00
110.00	0.00	7,471	965.40	0.00
115.00	0.00	7,470	965.40	0.00
120.00	0.00	7,470	965.40	0.00
125.00	0.00	7,470	965.40	0.00
130.00	0.00	7,469	965.40	0.00
135.00	0.00	7,469	965.40	0.00
140.00	0.00	7,469	965.40	0.00
145.00	0.00	7,469	965.40	0.00
150.00	0.00	7,468	965.40	0.00
155.00	0.00	7,468	965.40	0.00
160.00	0.00	7,468	965.40	0.00
165.00	0.00	7,468	965.40	0.00
170.00	0.00	7,468	965.40	0.00
175.00	0.00	7,468	965.40	0.00
180.00	0.00	7,468	965.40	0.00
185.00	0.00	7,468	965.40	0.00
190.00	0.00	7,468	965.40	0.00
195.00	0.00	7,468	965.40	0.00
200.00	0.00	7,468	965.40	0.00
205.00	0.00	7,468	965.40	0.00
210.00	0.00	7,468	965.40	0.00
215.00	0.00	7,468	965.40	0.00
220.00	0.00	7,468	965.40	0.00
225.00	0.00	7,468	965.40	0.00
230.00	0.00	7,468	965.40	0.00
235.00	0.00	7,468	965.40	0.00
240.00	0.00	7,468	965.40	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1.5" RAINFALL Rainfall=1.50"

Printed 6/5/2024

Page 30

Stage-Area-Storage for Pond 1P: UNDERGROUND POND 1

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
962.40	0	965.00	6,168	967.60	14,531
962.45	12	965.05	6,328	967.65	14,675
962.50	52	965.10	6,490	967.70	14,818
962.55	96	965.15	6,652	967.75	14,960
962.60	148	965.20	6,814	967.80	15,100
962.65	207	965.25	6,977	967.85	15,239
962.70	271	965.30	7,140	967.90	15,376
962.75	341	965.35	7,303	967.95	15,511
962.80	416	965.40	7,467	968.00	15,644
962.85	495	965.45	7,632	968.05	15,776
962.90	578	965.50	7,797	968.10	15,906
962.95	666	965.55	7,961	968.15	16,034
963.00	757	965.60	8,127	968.20	16,160
963.05	851	965.65	8,292	968.25	16,284
963.10	949	965.70	8,458	968.30	16,406
963.15	1,050	965.75	8,623	968.35	16,526
963.20	1,155	965.80	8,789	968.40	16,643
963.25	1,262	965.85	8,955	968.45	16,758
963.30	1,371	965.90	9,121	968.50	16,870
963.35	1,484	965.95	9,287	968.55	16,980
963.40	1,599	966.00	9,453	968.60	17,087
963.45	1,716	966.05	9,618	968.65	17,191
963.50	1,835	966.10	9,784	968.70	17,292
963.55	1,957	966.15	9,950	968.75	17,390
963.60	2,081	966.20	10,115	968.80	17,485
963.65	2,207	966.25	10,280	968.85	17,576
963.70	2,335	966.30	10,445	968.90	17,663
963.75	2,465	966.35	10,610	968.95	17,747
963.80	2,597	966.40	10,774	969.00	17,826
963.85	2,731	966.45	10,938	969.05	17,901
963.90	2,866	966.50	11,102	969.10	17,970
963.95	3,003	966.55	11,265	969.15	18,035
964.00	3,142	966.60	11,428	969.20	18,093
964.05	3,282	966.65	11,590	969.25	18,145
964.10	3,423	966.70	11,752	969.30	18,189
964.15	3,566	966.75	11,913	969.35	18,222
964.20	3,711	966.80	12,074	969.40	18,242
964.25	3,856	966.85	12,234	969.45	18,242
964.30	4,003	966.90	12,393	969.50	18,242
964.35	4,151	966.95	12,552	969.55	18,242
964.40	4,301	967.00	12,710	969.60	18,242
964.45	4,451	967.05	12,867	969.65	18,242
964.50	4,603	967.10	13,023	969.70	18,242
964.55	4,755	967.15	13,178	969.75	18,242
964.60	4,909	967.20	13,333	969.80	18,242
964.65	5,063	967.25	13,486	969.85	18,242
964.70	5,219	967.30	13,639	969.90	18,242
964.75	5,375	967.35	13,791		
964.80	5,532	967.40	13,941		
964.85	5,690	967.45	14,090		
964.90	5,849	967.50	14,238		
964.95	6,008	967.55	14,385		

24132 PROPOSED

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 31

Time span=0.00-240.00 hrs, dt=0.01 hrs, 24001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

SubcatchmentPR1A: DRAINS TO POND 1 Runoff Area=73,474 sf 83.48% Impervious Runoff Depth=2.04"
Tc=6.0 min CN=WQ Runoff=5.63 cfs 0.287 af

SubcatchmentPR1B: DRAINS SOUTH TO Runoff Area=14,493 sf 28.85% Impervious Runoff Depth=1.29"
Tc=6.0 min CN=WQ Runoff=0.74 cfs 0.036 af

SubcatchmentPR2: DRAINS NORTH TO Runoff Area=13,090 sf 21.37% Impervious Runoff Depth=1.18"
Tc=6.0 min CN=WQ Runoff=0.62 cfs 0.030 af

Pond 1P: UNDERGROUND POND 1 Peak Elev=966.93' Storage=12,475 cf Inflow=5.63 cfs 0.287 af
Outflow=2.02 cfs 0.287 af

Total Runoff Area = 2.320 ac Runoff Volume = 0.352 af Average Runoff Depth = 1.82"
32.40% Pervious = 0.752 ac 67.60% Impervious = 1.568 ac

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Printed 6/5/2024

Page 32

Summary for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Runoff = 5.63 cfs @ 12.13 hrs, Volume= 0.287 af, Depth= 2.04"
 Routed to Pond 1P : UNDERGROUND POND 1

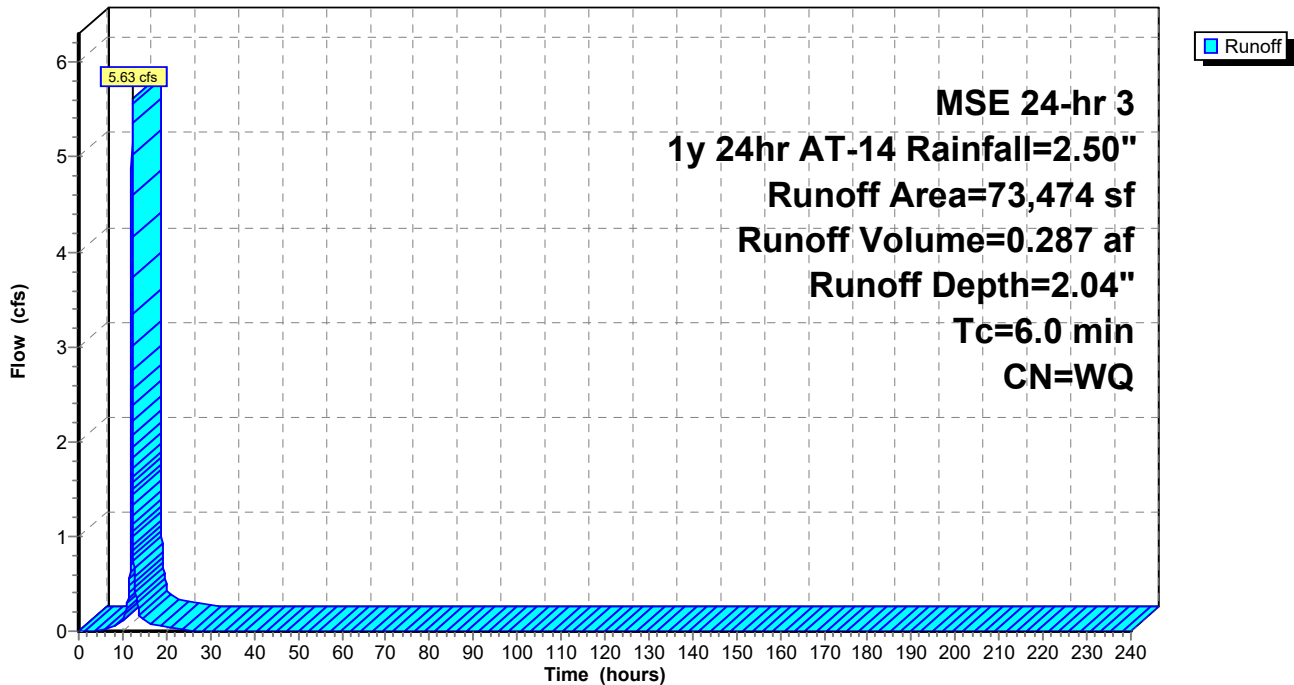
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Area (sf)	CN	Description
61,334	98	Paved parking, HSG D
12,140	80	>75% Grass cover, Good, HSG D
73,474		Weighted Average
12,140		16.52% Pervious Area
61,334		83.48% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Printed 6/5/2024

Page 33

Hydrograph for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.09	0.00	0.02
10.00	0.34	0.07	0.11
15.00	2.24	1.71	0.13
20.00	2.43	1.90	0.04
25.00	2.50	1.96	0.00
30.00	2.50	1.96	0.00
35.00	2.50	1.96	0.00
40.00	2.50	1.96	0.00
45.00	2.50	1.96	0.00
50.00	2.50	1.96	0.00
55.00	2.50	1.96	0.00
60.00	2.50	1.96	0.00
65.00	2.50	1.96	0.00
70.00	2.50	1.96	0.00
75.00	2.50	1.96	0.00
80.00	2.50	1.96	0.00
85.00	2.50	1.96	0.00
90.00	2.50	1.96	0.00
95.00	2.50	1.96	0.00
100.00	2.50	1.96	0.00
105.00	2.50	1.96	0.00
110.00	2.50	1.96	0.00
115.00	2.50	1.96	0.00
120.00	2.50	1.96	0.00
125.00	2.50	1.96	0.00
130.00	2.50	1.96	0.00
135.00	2.50	1.96	0.00
140.00	2.50	1.96	0.00
145.00	2.50	1.96	0.00
150.00	2.50	1.96	0.00
155.00	2.50	1.96	0.00
160.00	2.50	1.96	0.00
165.00	2.50	1.96	0.00
170.00	2.50	1.96	0.00
175.00	2.50	1.96	0.00
180.00	2.50	1.96	0.00
185.00	2.50	1.96	0.00
190.00	2.50	1.96	0.00
195.00	2.50	1.96	0.00
200.00	2.50	1.96	0.00
205.00	2.50	1.96	0.00
210.00	2.50	1.96	0.00
215.00	2.50	1.96	0.00
220.00	2.50	1.96	0.00
225.00	2.50	1.96	0.00
230.00	2.50	1.96	0.00
235.00	2.50	1.96	0.00
240.00	2.50	1.96	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Printed 6/5/2024

Page 34

Summary for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Runoff = 0.74 cfs @ 12.13 hrs, Volume= 0.036 af, Depth= 1.29"

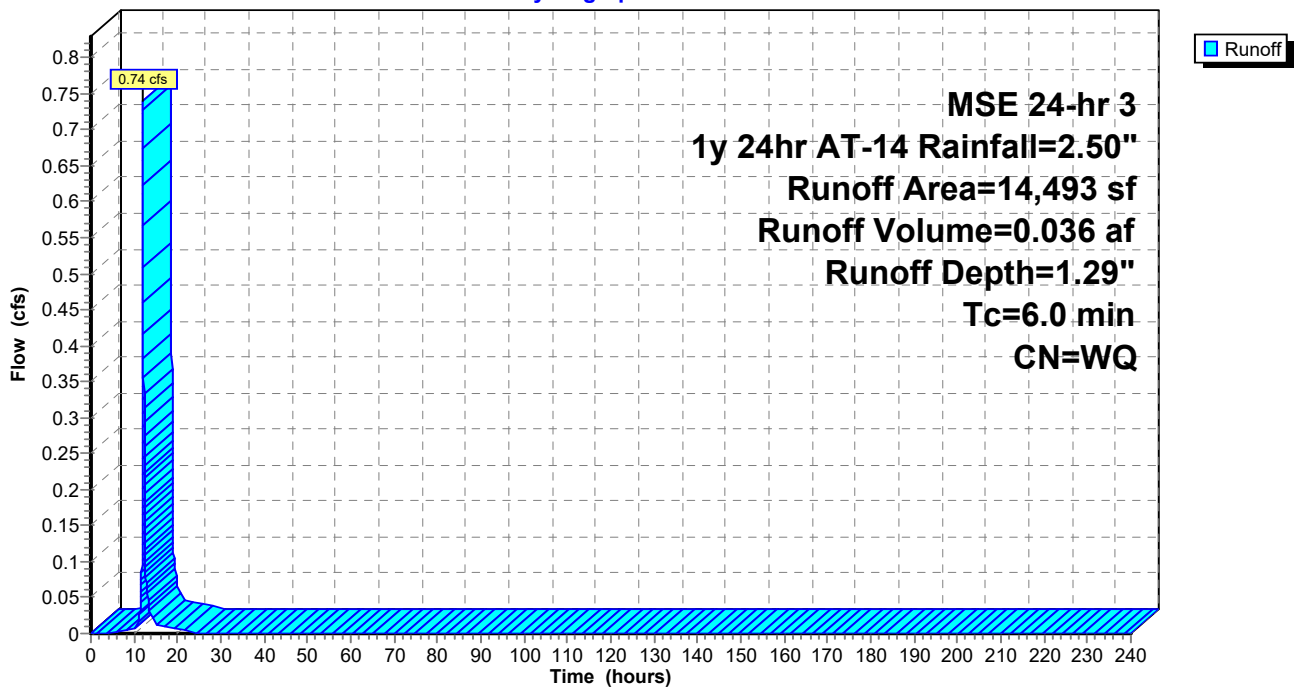
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Area (sf)	CN	Description
4,181	98	Paved parking, HSG D
10,312	80	>75% Grass cover, Good, HSG D
14,493		Weighted Average
10,312		71.15% Pervious Area
4,181		28.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Printed 6/5/2024

Page 35

Hydrograph for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.09	0.00	0.00
10.00	0.34	0.00	0.01
15.00	2.24	0.98	0.02
20.00	2.43	1.13	0.01
25.00	2.50	1.18	0.00
30.00	2.50	1.18	0.00
35.00	2.50	1.18	0.00
40.00	2.50	1.18	0.00
45.00	2.50	1.18	0.00
50.00	2.50	1.18	0.00
55.00	2.50	1.18	0.00
60.00	2.50	1.18	0.00
65.00	2.50	1.18	0.00
70.00	2.50	1.18	0.00
75.00	2.50	1.18	0.00
80.00	2.50	1.18	0.00
85.00	2.50	1.18	0.00
90.00	2.50	1.18	0.00
95.00	2.50	1.18	0.00
100.00	2.50	1.18	0.00
105.00	2.50	1.18	0.00
110.00	2.50	1.18	0.00
115.00	2.50	1.18	0.00
120.00	2.50	1.18	0.00
125.00	2.50	1.18	0.00
130.00	2.50	1.18	0.00
135.00	2.50	1.18	0.00
140.00	2.50	1.18	0.00
145.00	2.50	1.18	0.00
150.00	2.50	1.18	0.00
155.00	2.50	1.18	0.00
160.00	2.50	1.18	0.00
165.00	2.50	1.18	0.00
170.00	2.50	1.18	0.00
175.00	2.50	1.18	0.00
180.00	2.50	1.18	0.00
185.00	2.50	1.18	0.00
190.00	2.50	1.18	0.00
195.00	2.50	1.18	0.00
200.00	2.50	1.18	0.00
205.00	2.50	1.18	0.00
210.00	2.50	1.18	0.00
215.00	2.50	1.18	0.00
220.00	2.50	1.18	0.00
225.00	2.50	1.18	0.00
230.00	2.50	1.18	0.00
235.00	2.50	1.18	0.00
240.00	2.50	1.18	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Printed 6/5/2024

Page 36

Summary for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Runoff = 0.62 cfs @ 12.13 hrs, Volume= 0.030 af, Depth= 1.18"

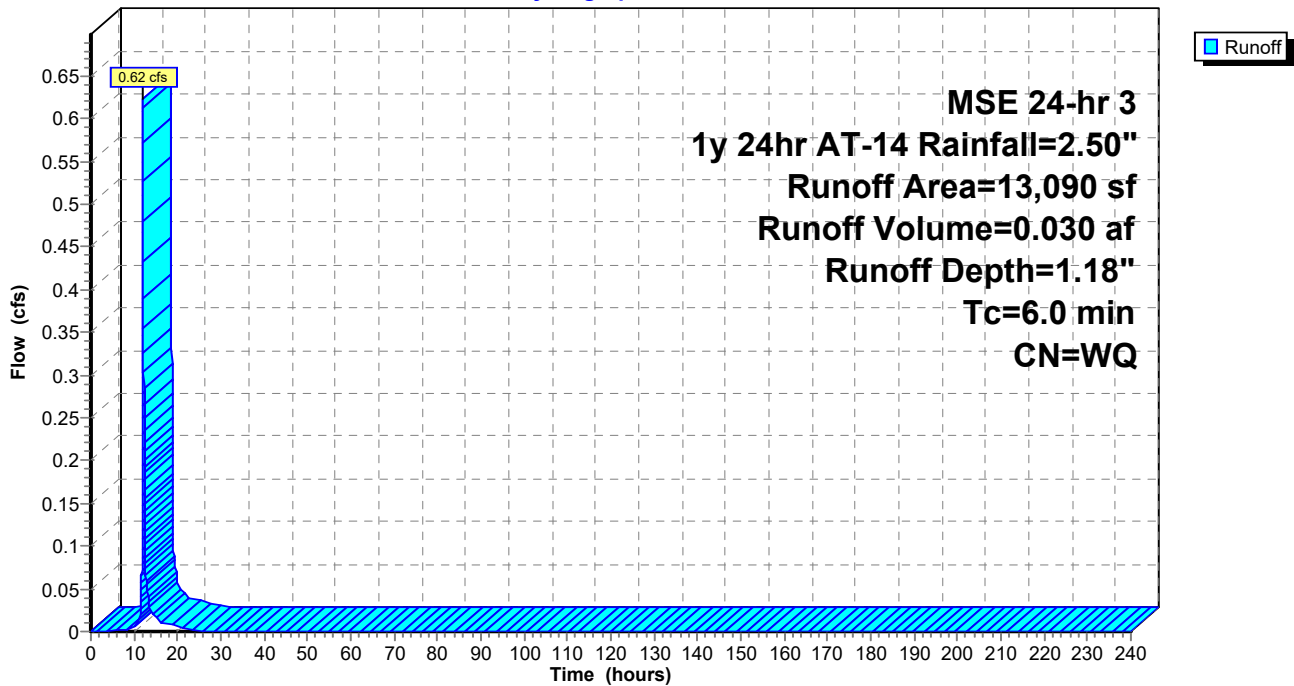
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Area (sf)	CN	Description
2,797	98	Paved parking, HSG D
10,293	80	>75% Grass cover, Good, HSG D
13,090		Weighted Average
10,293		78.63% Pervious Area
2,797		21.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Printed 6/5/2024

Page 37

Hydrograph for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.09	0.00	0.00
10.00	0.34	0.00	0.00
15.00	2.24	0.92	0.02
20.00	2.43	1.07	0.01
25.00	2.50	1.12	0.00
30.00	2.50	1.12	0.00
35.00	2.50	1.12	0.00
40.00	2.50	1.12	0.00
45.00	2.50	1.12	0.00
50.00	2.50	1.12	0.00
55.00	2.50	1.12	0.00
60.00	2.50	1.12	0.00
65.00	2.50	1.12	0.00
70.00	2.50	1.12	0.00
75.00	2.50	1.12	0.00
80.00	2.50	1.12	0.00
85.00	2.50	1.12	0.00
90.00	2.50	1.12	0.00
95.00	2.50	1.12	0.00
100.00	2.50	1.12	0.00
105.00	2.50	1.12	0.00
110.00	2.50	1.12	0.00
115.00	2.50	1.12	0.00
120.00	2.50	1.12	0.00
125.00	2.50	1.12	0.00
130.00	2.50	1.12	0.00
135.00	2.50	1.12	0.00
140.00	2.50	1.12	0.00
145.00	2.50	1.12	0.00
150.00	2.50	1.12	0.00
155.00	2.50	1.12	0.00
160.00	2.50	1.12	0.00
165.00	2.50	1.12	0.00
170.00	2.50	1.12	0.00
175.00	2.50	1.12	0.00
180.00	2.50	1.12	0.00
185.00	2.50	1.12	0.00
190.00	2.50	1.12	0.00
195.00	2.50	1.12	0.00
200.00	2.50	1.12	0.00
205.00	2.50	1.12	0.00
210.00	2.50	1.12	0.00
215.00	2.50	1.12	0.00
220.00	2.50	1.12	0.00
225.00	2.50	1.12	0.00
230.00	2.50	1.12	0.00
235.00	2.50	1.12	0.00
240.00	2.50	1.12	0.00

Summary for Pond 1P: UNDERGROUND POND 1

Inflow Area = 1.687 ac, 83.48% Impervious, Inflow Depth = 2.04" for 1y 24hr AT-14 event
 Inflow = 5.63 cfs @ 12.13 hrs, Volume= 0.287 af
 Outflow = 2.02 cfs @ 12.26 hrs, Volume= 0.287 af, Atten= 64%, Lag= 8.1 min
 Primary = 2.02 cfs @ 12.26 hrs, Volume= 0.287 af

Routing by Stor-Ind method, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 Starting Elev= 965.40' Surf.Area= 4,484 sf Storage= 7,467 cf
 Peak Elev= 966.93' @ 12.26 hrs Surf.Area= 4,484 sf Storage= 12,475 cf (5,007 cf above start)

Plug-Flow detention time= 376.1 min calculated for 0.116 af (40% of inflow)
 Center-of-Mass det. time= 96.4 min (857.4 - 761.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	962.40'	0 cf	19.00'W x 236.00'L x 7.50'H Field A 33,630 cf Overall - 18,242 cf Embedded = 15,388 cf x 0.0% Voids
#2A	962.40'	18,242 cf	CMP Round 84 x 22 Inside #1 Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf Overall Size= 84.0"W x 84.0"H x 20.00'L 22 Chambers in 2 Rows 17.00' Header x 38.48 sf x 2 = 1,308.5 cf Inside
		18,242 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	961.90'	15.0" Round Culvert L= 103.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 961.90' / 961.38' S= 0.0050 '/' Cc= 0.900 n= 0.012 Concrete pipe, finished, Flow Area= 1.23 sf
#2	Device 1	965.40'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	966.40'	16.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.02 cfs @ 12.26 hrs HW=966.93' (Free Discharge)

- ↑ **1=Culvert** (Passes 2.02 cfs of 10.54 cfs potential flow)
- ↑ **2=Orifice/Grate** (Orifice Controls 0.75 cfs @ 5.53 fps)
- ↑ **3=Orifice/Grate** (Orifice Controls 1.26 cfs @ 2.47 fps)

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Printed 6/5/2024

Page 39

Pond 1P: UNDERGROUND POND 1 - Chamber Wizard Field A

Chamber Model = CMP Round 84 (Round Corrugated Metal Pipe)

Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf

Overall Size= 84.0"W x 84.0"H x 20.00'L

84.0" Wide + 36.0" Spacing = 120.0" C-C Row Spacing

11 Chambers/Row x 20.00' Long +7.00' Header x 2 = 234.00' Row Length +12.0" End Stone x 2 = 236.00' Base Length

2 Rows x 84.0" Wide + 36.0" Spacing x 1 + 12.0" Side Stone x 2 = 19.00' Base Width

84.0" Chamber Height + 6.0" Stone Cover = 7.50' Field Height

22 Chambers x 769.7 cf + 17.00' Header x 38.48 sf x 2 = 18,241.7 cf Chamber Storage

33,630.0 cf Field - 18,241.7 cf Chambers = 15,388.3 cf Stone x 0.0% Voids = 0.0 cf Stone Storage

Chamber Storage = 18,241.7 cf = 0.419 af

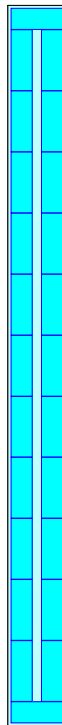
Overall Storage Efficiency = 54.2%

Overall System Size = 236.00' x 19.00' x 7.50'

22 Chambers

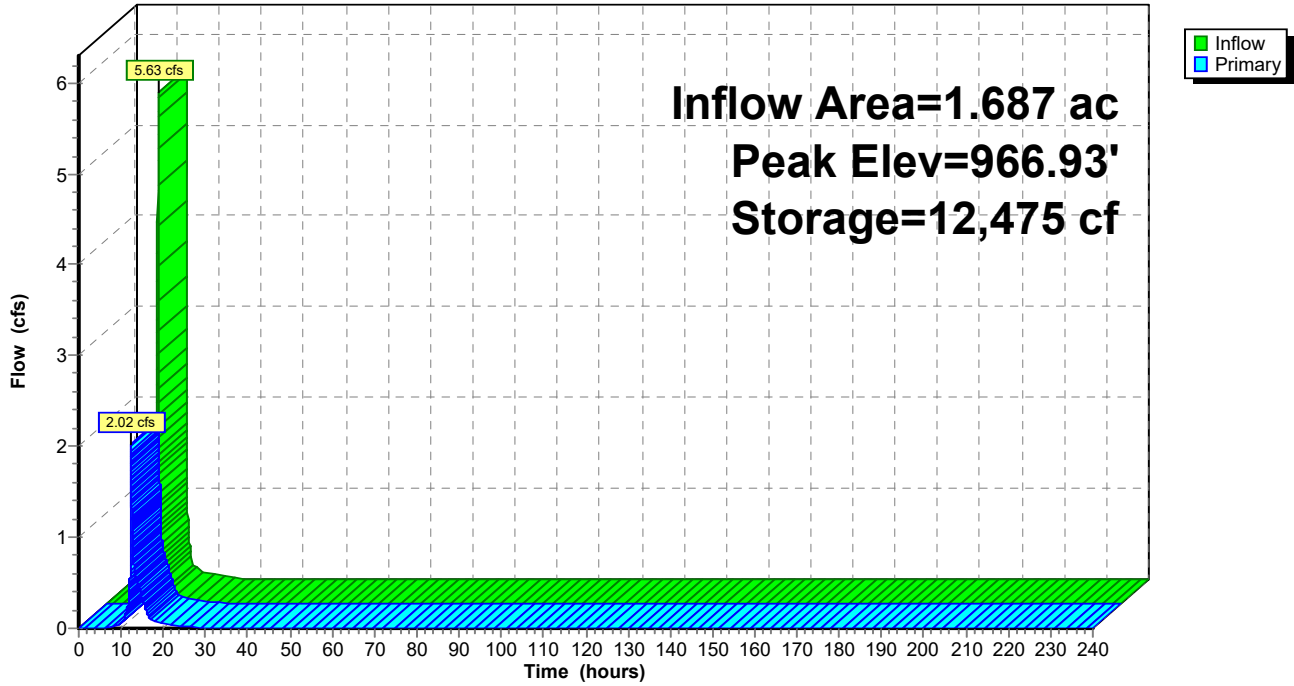
1,245.6 cy Field

569.9 cy Stone



Pond 1P: UNDERGROUND POND 1

Hydrograph



24132 PROPOSED

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 41

Hydrograph for Pond 1P: UNDERGROUND POND 1

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
0.00	0.00	7,467	965.40	0.00
5.00	0.02	7,516	965.41	0.00
10.00	0.11	7,987	965.56	0.06
15.00	0.13	9,009	965.87	0.33
20.00	0.04	7,941	965.54	0.05
25.00	0.00	7,702	965.47	0.01
30.00	0.00	7,553	965.43	0.00
35.00	0.00	7,512	965.41	0.00
40.00	0.00	7,501	965.41	0.00
45.00	0.00	7,496	965.41	0.00
50.00	0.00	7,492	965.41	0.00
55.00	0.00	7,488	965.41	0.00
60.00	0.00	7,485	965.41	0.00
65.00	0.00	7,482	965.40	0.00
70.00	0.00	7,480	965.40	0.00
75.00	0.00	7,478	965.40	0.00
80.00	0.00	7,477	965.40	0.00
85.00	0.00	7,475	965.40	0.00
90.00	0.00	7,474	965.40	0.00
95.00	0.00	7,473	965.40	0.00
100.00	0.00	7,472	965.40	0.00
105.00	0.00	7,472	965.40	0.00
110.00	0.00	7,471	965.40	0.00
115.00	0.00	7,471	965.40	0.00
120.00	0.00	7,470	965.40	0.00
125.00	0.00	7,470	965.40	0.00
130.00	0.00	7,469	965.40	0.00
135.00	0.00	7,469	965.40	0.00
140.00	0.00	7,469	965.40	0.00
145.00	0.00	7,469	965.40	0.00
150.00	0.00	7,468	965.40	0.00
155.00	0.00	7,468	965.40	0.00
160.00	0.00	7,468	965.40	0.00
165.00	0.00	7,468	965.40	0.00
170.00	0.00	7,468	965.40	0.00
175.00	0.00	7,468	965.40	0.00
180.00	0.00	7,468	965.40	0.00
185.00	0.00	7,468	965.40	0.00
190.00	0.00	7,468	965.40	0.00
195.00	0.00	7,468	965.40	0.00
200.00	0.00	7,468	965.40	0.00
205.00	0.00	7,468	965.40	0.00
210.00	0.00	7,468	965.40	0.00
215.00	0.00	7,468	965.40	0.00
220.00	0.00	7,468	965.40	0.00
225.00	0.00	7,468	965.40	0.00
230.00	0.00	7,468	965.40	0.00
235.00	0.00	7,468	965.40	0.00
240.00	0.00	7,468	965.40	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 1y 24hr AT-14 Rainfall=2.50"

Printed 6/5/2024

Page 42

Stage-Area-Storage for Pond 1P: UNDERGROUND POND 1

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
962.40	0	965.00	6,168	967.60	14,531
962.45	12	965.05	6,328	967.65	14,675
962.50	52	965.10	6,490	967.70	14,818
962.55	96	965.15	6,652	967.75	14,960
962.60	148	965.20	6,814	967.80	15,100
962.65	207	965.25	6,977	967.85	15,239
962.70	271	965.30	7,140	967.90	15,376
962.75	341	965.35	7,303	967.95	15,511
962.80	416	965.40	7,467	968.00	15,644
962.85	495	965.45	7,632	968.05	15,776
962.90	578	965.50	7,797	968.10	15,906
962.95	666	965.55	7,961	968.15	16,034
963.00	757	965.60	8,127	968.20	16,160
963.05	851	965.65	8,292	968.25	16,284
963.10	949	965.70	8,458	968.30	16,406
963.15	1,050	965.75	8,623	968.35	16,526
963.20	1,155	965.80	8,789	968.40	16,643
963.25	1,262	965.85	8,955	968.45	16,758
963.30	1,371	965.90	9,121	968.50	16,870
963.35	1,484	965.95	9,287	968.55	16,980
963.40	1,599	966.00	9,453	968.60	17,087
963.45	1,716	966.05	9,618	968.65	17,191
963.50	1,835	966.10	9,784	968.70	17,292
963.55	1,957	966.15	9,950	968.75	17,390
963.60	2,081	966.20	10,115	968.80	17,485
963.65	2,207	966.25	10,280	968.85	17,576
963.70	2,335	966.30	10,445	968.90	17,663
963.75	2,465	966.35	10,610	968.95	17,747
963.80	2,597	966.40	10,774	969.00	17,826
963.85	2,731	966.45	10,938	969.05	17,901
963.90	2,866	966.50	11,102	969.10	17,970
963.95	3,003	966.55	11,265	969.15	18,035
964.00	3,142	966.60	11,428	969.20	18,093
964.05	3,282	966.65	11,590	969.25	18,145
964.10	3,423	966.70	11,752	969.30	18,189
964.15	3,566	966.75	11,913	969.35	18,222
964.20	3,711	966.80	12,074	969.40	18,242
964.25	3,856	966.85	12,234	969.45	18,242
964.30	4,003	966.90	12,393	969.50	18,242
964.35	4,151	966.95	12,552	969.55	18,242
964.40	4,301	967.00	12,710	969.60	18,242
964.45	4,451	967.05	12,867	969.65	18,242
964.50	4,603	967.10	13,023	969.70	18,242
964.55	4,755	967.15	13,178	969.75	18,242
964.60	4,909	967.20	13,333	969.80	18,242
964.65	5,063	967.25	13,486	969.85	18,242
964.70	5,219	967.30	13,639	969.90	18,242
964.75	5,375	967.35	13,791		
964.80	5,532	967.40	13,941		
964.85	5,690	967.45	14,090		
964.90	5,849	967.50	14,238		
964.95	6,008	967.55	14,385		

24132 PROPOSED

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 43

Time span=0.00-240.00 hrs, dt=0.01 hrs, 24001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

SubcatchmentPR1A: DRAINS TO POND 1 Runoff Area=73,474 sf 83.48% Impervious Runoff Depth=2.40"
Tc=6.0 min CN=WQ Runoff=6.58 cfs 0.338 af

SubcatchmentPR1B: DRAINS SOUTH TO Runoff Area=14,493 sf 28.85% Impervious Runoff Depth=1.59"
Tc=6.0 min CN=WQ Runoff=0.92 cfs 0.044 af

SubcatchmentPR2: DRAINS NORTH TO Runoff Area=13,090 sf 21.37% Impervious Runoff Depth=1.48"
Tc=6.0 min CN=WQ Runoff=0.78 cfs 0.037 af

Pond 1P: UNDERGROUND POND 1 Peak Elev=967.10' Storage=13,012 cf Inflow=6.58 cfs 0.338 af
Outflow=2.90 cfs 0.338 af

Total Runoff Area = 2.320 ac Runoff Volume = 0.419 af Average Runoff Depth = 2.17"
32.40% Pervious = 0.752 ac 67.60% Impervious = 1.568 ac

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 44

Summary for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Runoff = 6.58 cfs @ 12.13 hrs, Volume= 0.338 af, Depth= 2.40"
Routed to Pond 1P : UNDERGROUND POND 1

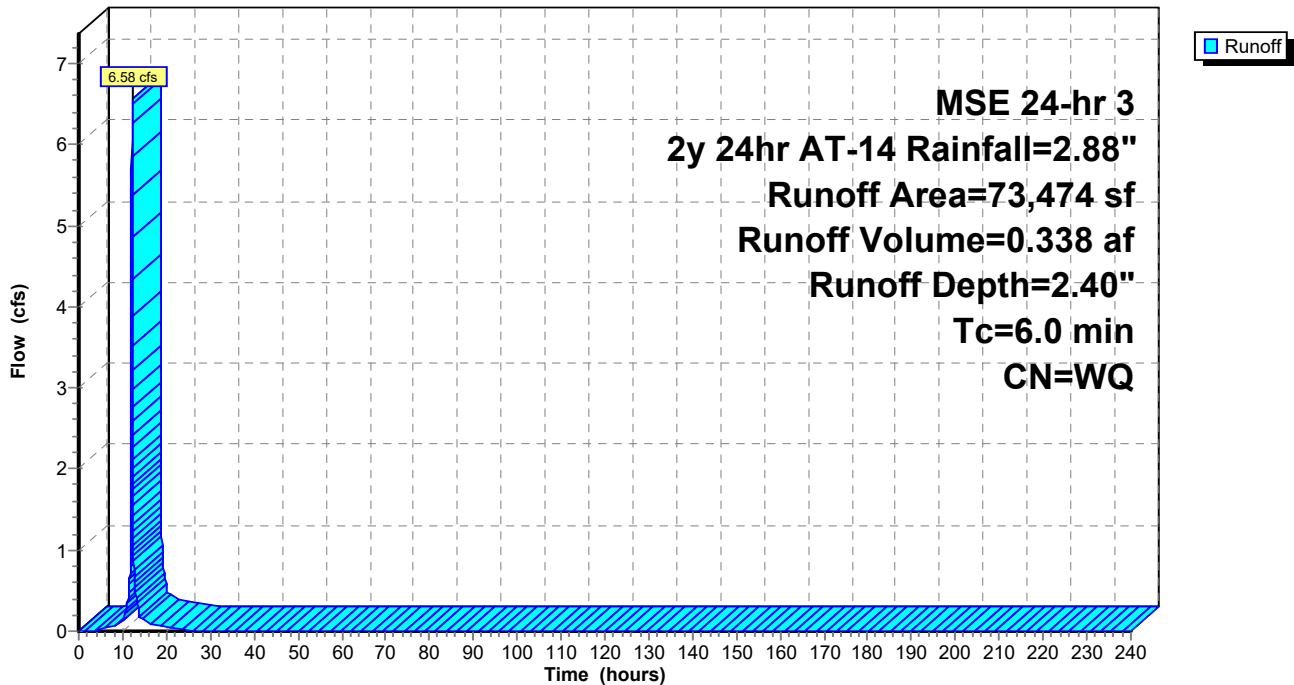
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Area (sf)	CN	Description
61,334	98	Paved parking, HSG D
12,140	80	>75% Grass cover, Good, HSG D
73,474		Weighted Average
12,140		16.52% Pervious Area
61,334		83.48% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 45

Hydrograph for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.11	0.00	0.02
10.00	0.40	0.10	0.13
15.00	2.58	2.04	0.15
20.00	2.80	2.26	0.05
25.00	2.88	2.33	0.00
30.00	2.88	2.33	0.00
35.00	2.88	2.33	0.00
40.00	2.88	2.33	0.00
45.00	2.88	2.33	0.00
50.00	2.88	2.33	0.00
55.00	2.88	2.33	0.00
60.00	2.88	2.33	0.00
65.00	2.88	2.33	0.00
70.00	2.88	2.33	0.00
75.00	2.88	2.33	0.00
80.00	2.88	2.33	0.00
85.00	2.88	2.33	0.00
90.00	2.88	2.33	0.00
95.00	2.88	2.33	0.00
100.00	2.88	2.33	0.00
105.00	2.88	2.33	0.00
110.00	2.88	2.33	0.00
115.00	2.88	2.33	0.00
120.00	2.88	2.33	0.00
125.00	2.88	2.33	0.00
130.00	2.88	2.33	0.00
135.00	2.88	2.33	0.00
140.00	2.88	2.33	0.00
145.00	2.88	2.33	0.00
150.00	2.88	2.33	0.00
155.00	2.88	2.33	0.00
160.00	2.88	2.33	0.00
165.00	2.88	2.33	0.00
170.00	2.88	2.33	0.00
175.00	2.88	2.33	0.00
180.00	2.88	2.33	0.00
185.00	2.88	2.33	0.00
190.00	2.88	2.33	0.00
195.00	2.88	2.33	0.00
200.00	2.88	2.33	0.00
205.00	2.88	2.33	0.00
210.00	2.88	2.33	0.00
215.00	2.88	2.33	0.00
220.00	2.88	2.33	0.00
225.00	2.88	2.33	0.00
230.00	2.88	2.33	0.00
235.00	2.88	2.33	0.00
240.00	2.88	2.33	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 46

Summary for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Runoff = 0.92 cfs @ 12.13 hrs, Volume= 0.044 af, Depth= 1.59"

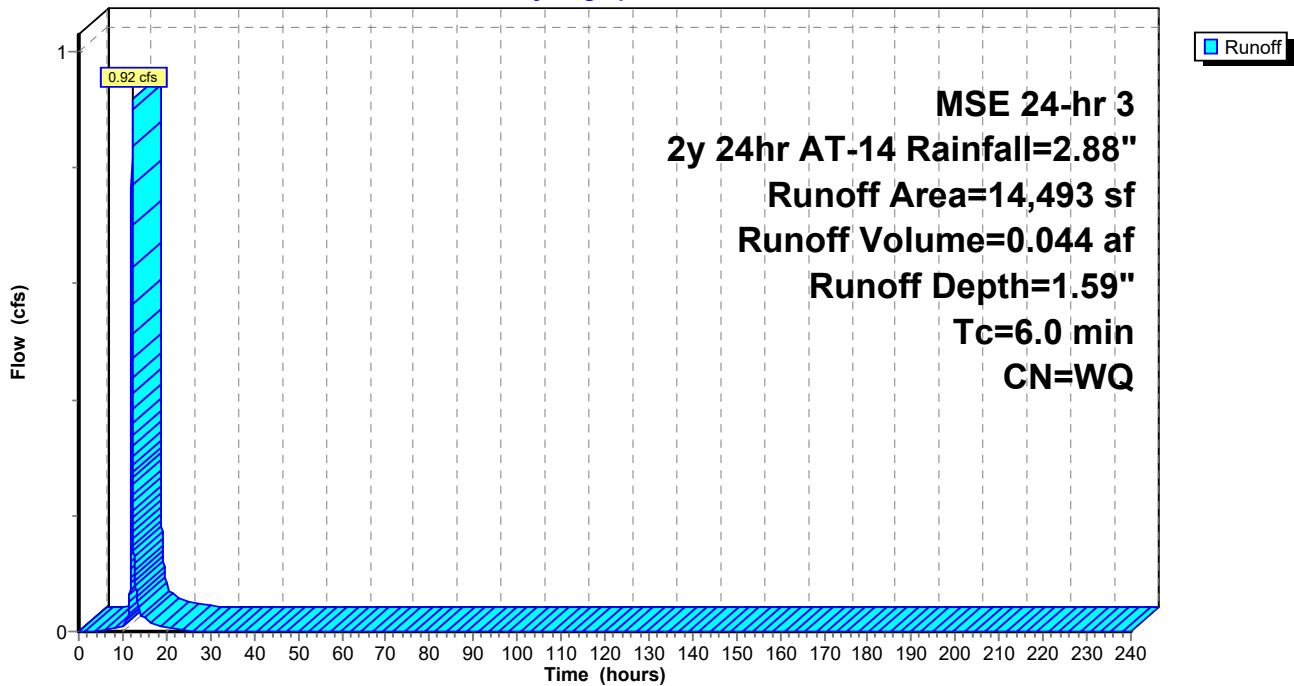
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Area (sf)	CN	Description
4,181	98	Paved parking, HSG D
10,312	80	>75% Grass cover, Good, HSG D
14,493		Weighted Average
10,312		71.15% Pervious Area
4,181		28.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 47

Hydrograph for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.11	0.00	0.00
10.00	0.40	0.00	0.01
15.00	2.58	1.25	0.02
20.00	2.80	1.43	0.01
25.00	2.88	1.49	0.00
30.00	2.88	1.49	0.00
35.00	2.88	1.49	0.00
40.00	2.88	1.49	0.00
45.00	2.88	1.49	0.00
50.00	2.88	1.49	0.00
55.00	2.88	1.49	0.00
60.00	2.88	1.49	0.00
65.00	2.88	1.49	0.00
70.00	2.88	1.49	0.00
75.00	2.88	1.49	0.00
80.00	2.88	1.49	0.00
85.00	2.88	1.49	0.00
90.00	2.88	1.49	0.00
95.00	2.88	1.49	0.00
100.00	2.88	1.49	0.00
105.00	2.88	1.49	0.00
110.00	2.88	1.49	0.00
115.00	2.88	1.49	0.00
120.00	2.88	1.49	0.00
125.00	2.88	1.49	0.00
130.00	2.88	1.49	0.00
135.00	2.88	1.49	0.00
140.00	2.88	1.49	0.00
145.00	2.88	1.49	0.00
150.00	2.88	1.49	0.00
155.00	2.88	1.49	0.00
160.00	2.88	1.49	0.00
165.00	2.88	1.49	0.00
170.00	2.88	1.49	0.00
175.00	2.88	1.49	0.00
180.00	2.88	1.49	0.00
185.00	2.88	1.49	0.00
190.00	2.88	1.49	0.00
195.00	2.88	1.49	0.00
200.00	2.88	1.49	0.00
205.00	2.88	1.49	0.00
210.00	2.88	1.49	0.00
215.00	2.88	1.49	0.00
220.00	2.88	1.49	0.00
225.00	2.88	1.49	0.00
230.00	2.88	1.49	0.00
235.00	2.88	1.49	0.00
240.00	2.88	1.49	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 48

Summary for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Runoff = 0.78 cfs @ 12.13 hrs, Volume= 0.037 af, Depth= 1.48"

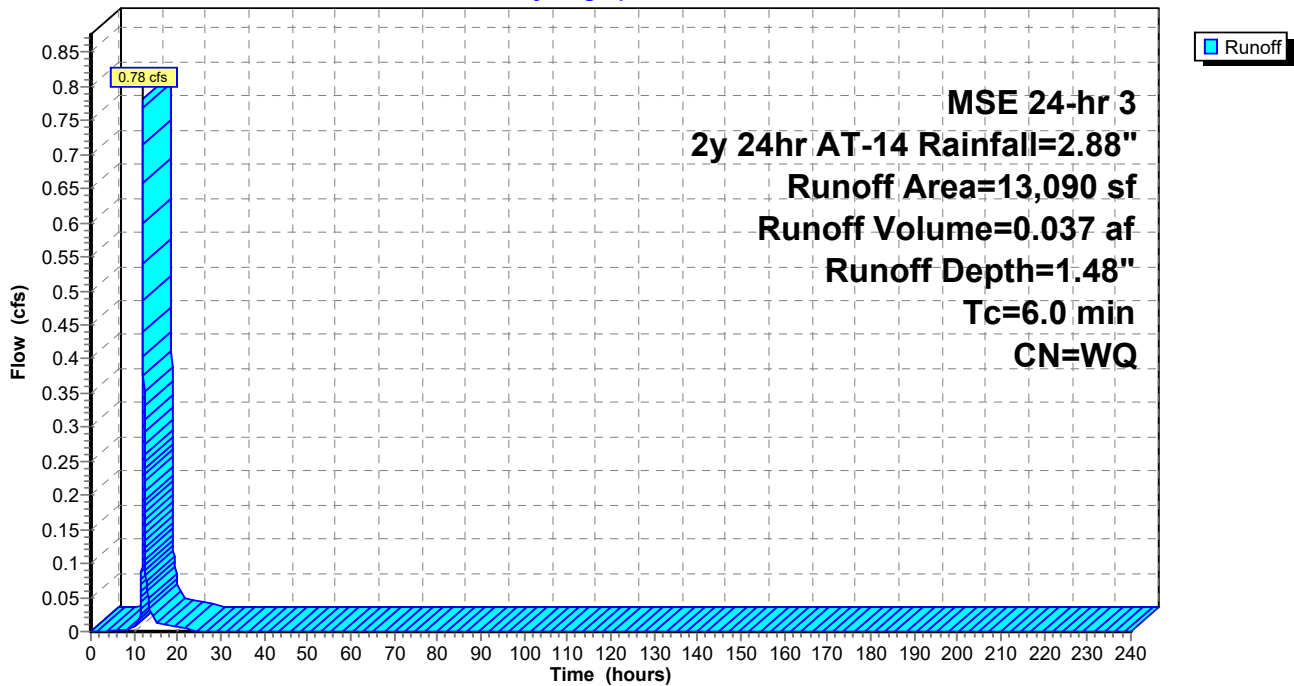
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Area (sf)	CN	Description
2,797	98	Paved parking, HSG D
10,293	80	>75% Grass cover, Good, HSG D
13,090		Weighted Average
10,293		78.63% Pervious Area
2,797		21.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 49

Hydrograph for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.11	0.00	0.00
10.00	0.40	0.00	0.01
15.00	2.58	1.18	0.02
20.00	2.80	1.36	0.01
25.00	2.88	1.42	0.00
30.00	2.88	1.42	0.00
35.00	2.88	1.42	0.00
40.00	2.88	1.42	0.00
45.00	2.88	1.42	0.00
50.00	2.88	1.42	0.00
55.00	2.88	1.42	0.00
60.00	2.88	1.42	0.00
65.00	2.88	1.42	0.00
70.00	2.88	1.42	0.00
75.00	2.88	1.42	0.00
80.00	2.88	1.42	0.00
85.00	2.88	1.42	0.00
90.00	2.88	1.42	0.00
95.00	2.88	1.42	0.00
100.00	2.88	1.42	0.00
105.00	2.88	1.42	0.00
110.00	2.88	1.42	0.00
115.00	2.88	1.42	0.00
120.00	2.88	1.42	0.00
125.00	2.88	1.42	0.00
130.00	2.88	1.42	0.00
135.00	2.88	1.42	0.00
140.00	2.88	1.42	0.00
145.00	2.88	1.42	0.00
150.00	2.88	1.42	0.00
155.00	2.88	1.42	0.00
160.00	2.88	1.42	0.00
165.00	2.88	1.42	0.00
170.00	2.88	1.42	0.00
175.00	2.88	1.42	0.00
180.00	2.88	1.42	0.00
185.00	2.88	1.42	0.00
190.00	2.88	1.42	0.00
195.00	2.88	1.42	0.00
200.00	2.88	1.42	0.00
205.00	2.88	1.42	0.00
210.00	2.88	1.42	0.00
215.00	2.88	1.42	0.00
220.00	2.88	1.42	0.00
225.00	2.88	1.42	0.00
230.00	2.88	1.42	0.00
235.00	2.88	1.42	0.00
240.00	2.88	1.42	0.00

24132 PROPOSED

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 50

Summary for Pond 1P: UNDERGROUND POND 1

Inflow Area = 1.687 ac, 83.48% Impervious, Inflow Depth = 2.40" for 2y 24hr AT-14 event
 Inflow = 6.58 cfs @ 12.13 hrs, Volume= 0.338 af
 Outflow = 2.90 cfs @ 12.23 hrs, Volume= 0.338 af, Atten= 56%, Lag= 6.3 min
 Primary = 2.90 cfs @ 12.23 hrs, Volume= 0.338 af

Routing by Stor-Ind method, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 Starting Elev= 965.40' Surf.Area= 4,484 sf Storage= 7,467 cf
 Peak Elev= 967.10' @ 12.23 hrs Surf.Area= 4,484 sf Storage= 13,012 cf (5,545 cf above start)

Plug-Flow detention time= 312.8 min calculated for 0.166 af (49% of inflow)
 Center-of-Mass det. time= 88.9 min (847.6 - 758.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	962.40'	0 cf	19.00'W x 236.00'L x 7.50'H Field A 33,630 cf Overall - 18,242 cf Embedded = 15,388 cf x 0.0% Voids
#2A	962.40'	18,242 cf	CMP Round 84 x 22 Inside #1 Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf Overall Size= 84.0"W x 84.0"H x 20.00'L 22 Chambers in 2 Rows 17.00' Header x 38.48 sf x 2 = 1,308.5 cf Inside
		18,242 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	961.90'	15.0" Round Culvert L= 103.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 961.90' / 961.38' S= 0.0050 '/ Cc= 0.900 n= 0.012 Concrete pipe, finished, Flow Area= 1.23 sf
#2	Device 1	965.40'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	966.40'	16.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.90 cfs @ 12.23 hrs HW=967.10' (Free Discharge)

- ↑ **1=Culvert** (Passes 2.90 cfs of 10.75 cfs potential flow)
- ↑ **2=Orifice/Grate** (Orifice Controls 0.80 cfs @ 5.87 fps)
- ↑ **3=Orifice/Grate** (Orifice Controls 2.10 cfs @ 2.84 fps)

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 51

Pond 1P: UNDERGROUND POND 1 - Chamber Wizard Field A

Chamber Model = CMP Round 84 (Round Corrugated Metal Pipe)

Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf

Overall Size= 84.0"W x 84.0"H x 20.00'L

84.0" Wide + 36.0" Spacing = 120.0" C-C Row Spacing

11 Chambers/Row x 20.00' Long +7.00' Header x 2 = 234.00' Row Length +12.0" End Stone x 2 = 236.00' Base Length

2 Rows x 84.0" Wide + 36.0" Spacing x 1 + 12.0" Side Stone x 2 = 19.00' Base Width

84.0" Chamber Height + 6.0" Stone Cover = 7.50' Field Height

22 Chambers x 769.7 cf + 17.00' Header x 38.48 sf x 2 = 18,241.7 cf Chamber Storage

33,630.0 cf Field - 18,241.7 cf Chambers = 15,388.3 cf Stone x 0.0% Voids = 0.0 cf Stone Storage

Chamber Storage = 18,241.7 cf = 0.419 af

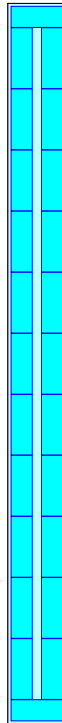
Overall Storage Efficiency = 54.2%

Overall System Size = 236.00' x 19.00' x 7.50'

22 Chambers

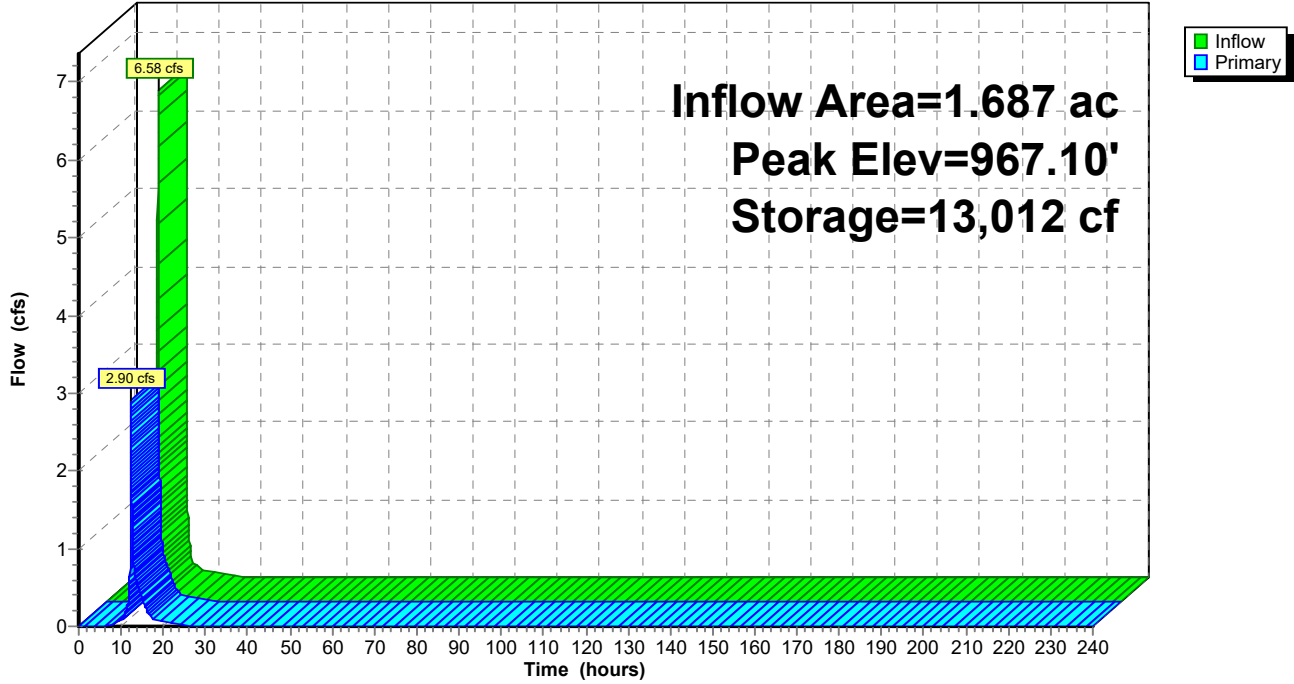
1,245.6 cy Field

569.9 cy Stone



Pond 1P: UNDERGROUND POND 1

Hydrograph



24132 PROPOSED

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 53

Hydrograph for Pond 1P: UNDERGROUND POND 1

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
0.00	0.00	7,467	965.40	0.00
5.00	0.02	7,541	965.42	0.00
10.00	0.13	8,063	965.58	0.08
15.00	0.15	9,162	965.91	0.36
20.00	0.05	7,976	965.55	0.06
25.00	0.00	7,714	965.47	0.02
30.00	0.00	7,556	965.43	0.00
35.00	0.00	7,513	965.41	0.00
40.00	0.00	7,501	965.41	0.00
45.00	0.00	7,496	965.41	0.00
50.00	0.00	7,492	965.41	0.00
55.00	0.00	7,488	965.41	0.00
60.00	0.00	7,485	965.41	0.00
65.00	0.00	7,483	965.40	0.00
70.00	0.00	7,480	965.40	0.00
75.00	0.00	7,478	965.40	0.00
80.00	0.00	7,477	965.40	0.00
85.00	0.00	7,475	965.40	0.00
90.00	0.00	7,474	965.40	0.00
95.00	0.00	7,473	965.40	0.00
100.00	0.00	7,472	965.40	0.00
105.00	0.00	7,472	965.40	0.00
110.00	0.00	7,471	965.40	0.00
115.00	0.00	7,471	965.40	0.00
120.00	0.00	7,470	965.40	0.00
125.00	0.00	7,470	965.40	0.00
130.00	0.00	7,469	965.40	0.00
135.00	0.00	7,469	965.40	0.00
140.00	0.00	7,469	965.40	0.00
145.00	0.00	7,469	965.40	0.00
150.00	0.00	7,468	965.40	0.00
155.00	0.00	7,468	965.40	0.00
160.00	0.00	7,468	965.40	0.00
165.00	0.00	7,468	965.40	0.00
170.00	0.00	7,468	965.40	0.00
175.00	0.00	7,468	965.40	0.00
180.00	0.00	7,468	965.40	0.00
185.00	0.00	7,468	965.40	0.00
190.00	0.00	7,468	965.40	0.00
195.00	0.00	7,468	965.40	0.00
200.00	0.00	7,468	965.40	0.00
205.00	0.00	7,468	965.40	0.00
210.00	0.00	7,468	965.40	0.00
215.00	0.00	7,468	965.40	0.00
220.00	0.00	7,468	965.40	0.00
225.00	0.00	7,468	965.40	0.00
230.00	0.00	7,468	965.40	0.00
235.00	0.00	7,468	965.40	0.00
240.00	0.00	7,468	965.40	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 2y 24hr AT-14 Rainfall=2.88"

Printed 6/5/2024

Page 54

Stage-Area-Storage for Pond 1P: UNDERGROUND POND 1

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
962.40	0	965.00	6,168	967.60	14,531
962.45	12	965.05	6,328	967.65	14,675
962.50	52	965.10	6,490	967.70	14,818
962.55	96	965.15	6,652	967.75	14,960
962.60	148	965.20	6,814	967.80	15,100
962.65	207	965.25	6,977	967.85	15,239
962.70	271	965.30	7,140	967.90	15,376
962.75	341	965.35	7,303	967.95	15,511
962.80	416	965.40	7,467	968.00	15,644
962.85	495	965.45	7,632	968.05	15,776
962.90	578	965.50	7,797	968.10	15,906
962.95	666	965.55	7,961	968.15	16,034
963.00	757	965.60	8,127	968.20	16,160
963.05	851	965.65	8,292	968.25	16,284
963.10	949	965.70	8,458	968.30	16,406
963.15	1,050	965.75	8,623	968.35	16,526
963.20	1,155	965.80	8,789	968.40	16,643
963.25	1,262	965.85	8,955	968.45	16,758
963.30	1,371	965.90	9,121	968.50	16,870
963.35	1,484	965.95	9,287	968.55	16,980
963.40	1,599	966.00	9,453	968.60	17,087
963.45	1,716	966.05	9,618	968.65	17,191
963.50	1,835	966.10	9,784	968.70	17,292
963.55	1,957	966.15	9,950	968.75	17,390
963.60	2,081	966.20	10,115	968.80	17,485
963.65	2,207	966.25	10,280	968.85	17,576
963.70	2,335	966.30	10,445	968.90	17,663
963.75	2,465	966.35	10,610	968.95	17,747
963.80	2,597	966.40	10,774	969.00	17,826
963.85	2,731	966.45	10,938	969.05	17,901
963.90	2,866	966.50	11,102	969.10	17,970
963.95	3,003	966.55	11,265	969.15	18,035
964.00	3,142	966.60	11,428	969.20	18,093
964.05	3,282	966.65	11,590	969.25	18,145
964.10	3,423	966.70	11,752	969.30	18,189
964.15	3,566	966.75	11,913	969.35	18,222
964.20	3,711	966.80	12,074	969.40	18,242
964.25	3,856	966.85	12,234	969.45	18,242
964.30	4,003	966.90	12,393	969.50	18,242
964.35	4,151	966.95	12,552	969.55	18,242
964.40	4,301	967.00	12,710	969.60	18,242
964.45	4,451	967.05	12,867	969.65	18,242
964.50	4,603	967.10	13,023	969.70	18,242
964.55	4,755	967.15	13,178	969.75	18,242
964.60	4,909	967.20	13,333	969.80	18,242
964.65	5,063	967.25	13,486	969.85	18,242
964.70	5,219	967.30	13,639	969.90	18,242
964.75	5,375	967.35	13,791		
964.80	5,532	967.40	13,941		
964.85	5,690	967.45	14,090		
964.90	5,849	967.50	14,238		
964.95	6,008	967.55	14,385		

24132 PROPOSED

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 55

Time span=0.00-240.00 hrs, dt=0.01 hrs, 24001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

SubcatchmentPR1A: DRAINS TO POND 1 Runoff Area=73,474 sf 83.48% Impervious Runoff Depth=3.76"
Tc=6.0 min CN=WQ Runoff=10.16 cfs 0.529 af

SubcatchmentPR1B: DRAINS SOUTH TO Runoff Area=14,493 sf 28.85% Impervious Runoff Depth=2.79"
Tc=6.0 min CN=WQ Runoff=1.61 cfs 0.077 af

SubcatchmentPR2: DRAINS NORTH TO Runoff Area=13,090 sf 21.37% Impervious Runoff Depth=2.66"
Tc=6.0 min CN=WQ Runoff=1.41 cfs 0.067 af

Pond 1P: UNDERGROUND POND 1 Peak Elev=967.67' Storage=14,720 cf Inflow=10.16 cfs 0.529 af
Outflow=6.18 cfs 0.529 af

Total Runoff Area = 2.320 ac Runoff Volume = 0.673 af Average Runoff Depth = 3.48"
32.40% Pervious = 0.752 ac 67.60% Impervious = 1.568 ac

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 56

Summary for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Runoff = 10.16 cfs @ 12.13 hrs, Volume= 0.529 af, Depth= 3.76"
 Routed to Pond 1P : UNDERGROUND POND 1

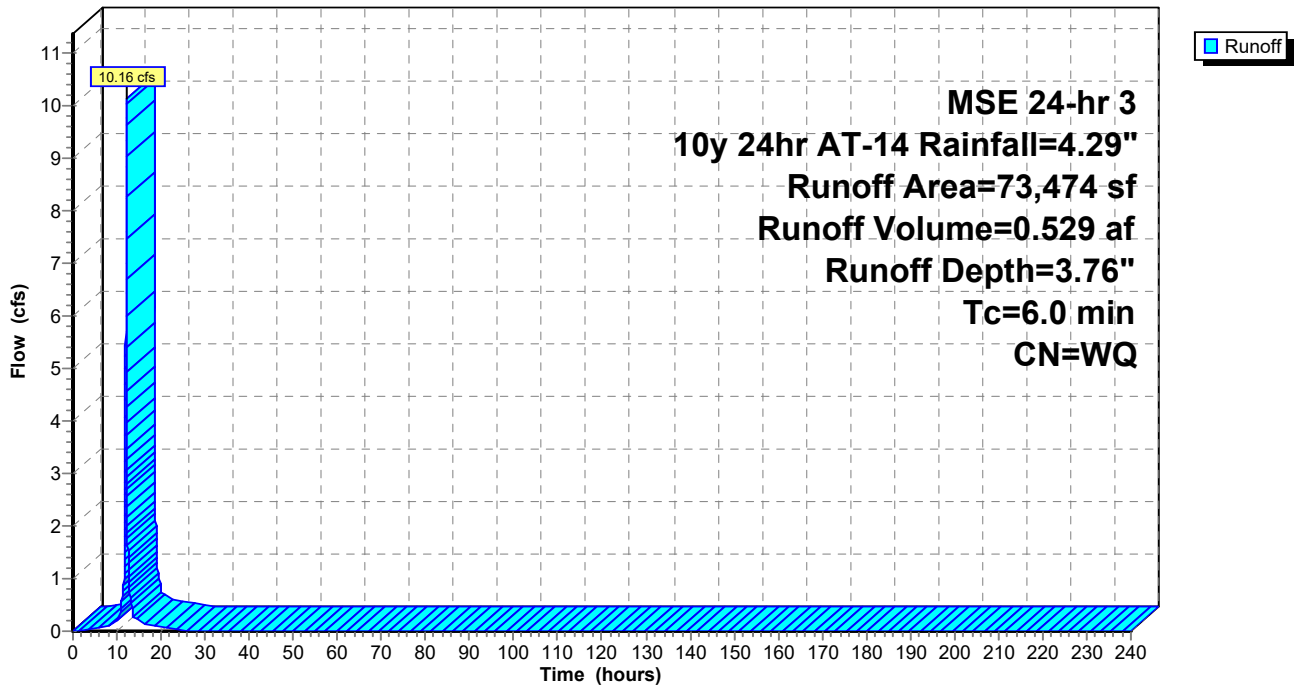
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Area (sf)	CN	Description
61,334	98	Paved parking, HSG D
12,140	80	>75% Grass cover, Good, HSG D
73,474		Weighted Average
12,140		16.52% Pervious Area
61,334		83.48% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 57

Hydrograph for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.16	0.01	0.04
10.00	0.59	0.23	0.21
15.00	3.85	3.28	0.23
20.00	4.18	3.61	0.08
25.00	4.29	3.72	0.00
30.00	4.29	3.72	0.00
35.00	4.29	3.72	0.00
40.00	4.29	3.72	0.00
45.00	4.29	3.72	0.00
50.00	4.29	3.72	0.00
55.00	4.29	3.72	0.00
60.00	4.29	3.72	0.00
65.00	4.29	3.72	0.00
70.00	4.29	3.72	0.00
75.00	4.29	3.72	0.00
80.00	4.29	3.72	0.00
85.00	4.29	3.72	0.00
90.00	4.29	3.72	0.00
95.00	4.29	3.72	0.00
100.00	4.29	3.72	0.00
105.00	4.29	3.72	0.00
110.00	4.29	3.72	0.00
115.00	4.29	3.72	0.00
120.00	4.29	3.72	0.00
125.00	4.29	3.72	0.00
130.00	4.29	3.72	0.00
135.00	4.29	3.72	0.00
140.00	4.29	3.72	0.00
145.00	4.29	3.72	0.00
150.00	4.29	3.72	0.00
155.00	4.29	3.72	0.00
160.00	4.29	3.72	0.00
165.00	4.29	3.72	0.00
170.00	4.29	3.72	0.00
175.00	4.29	3.72	0.00
180.00	4.29	3.72	0.00
185.00	4.29	3.72	0.00
190.00	4.29	3.72	0.00
195.00	4.29	3.72	0.00
200.00	4.29	3.72	0.00
205.00	4.29	3.72	0.00
210.00	4.29	3.72	0.00
215.00	4.29	3.72	0.00
220.00	4.29	3.72	0.00
225.00	4.29	3.72	0.00
230.00	4.29	3.72	0.00
235.00	4.29	3.72	0.00
240.00	4.29	3.72	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 58

Summary for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Runoff = 1.61 cfs @ 12.13 hrs, Volume= 0.077 af, Depth= 2.79"

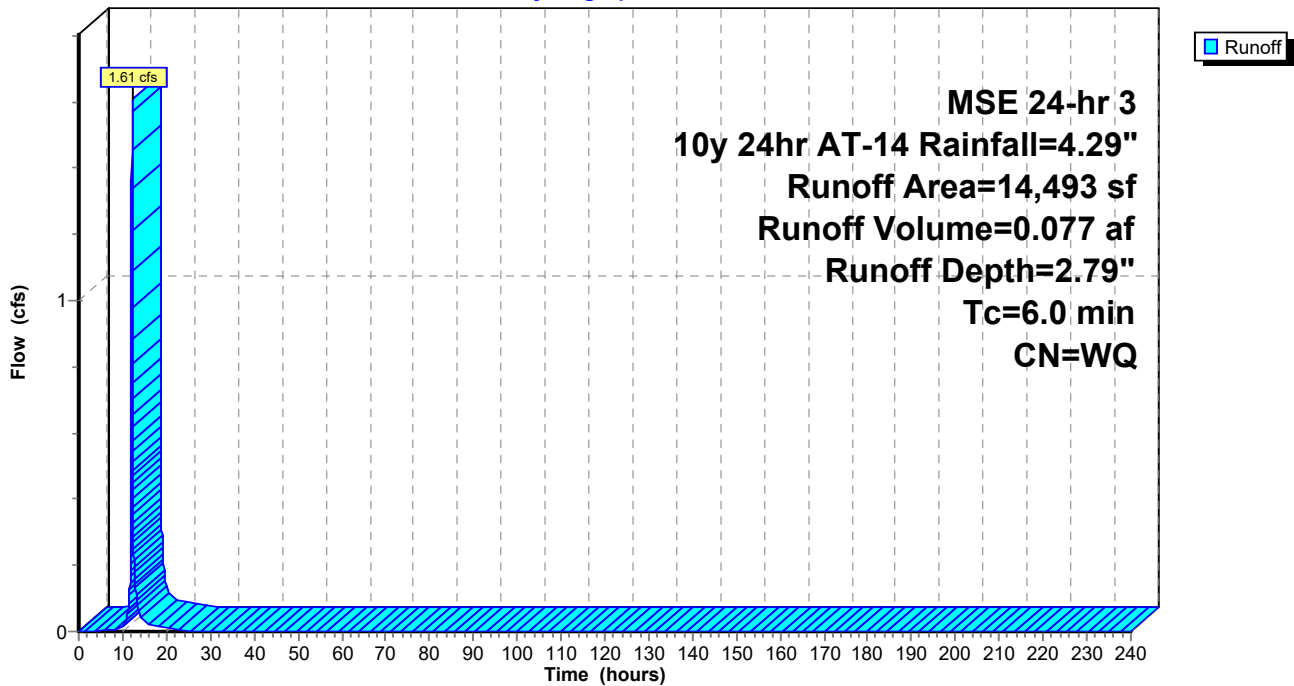
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Area (sf)	CN	Description
4,181	98	Paved parking, HSG D
10,312	80	>75% Grass cover, Good, HSG D
14,493		Weighted Average
10,312		71.15% Pervious Area
4,181		28.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 59

Hydrograph for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.16	0.00	0.00
10.00	0.59	0.03	0.02
15.00	3.85	2.32	0.04
20.00	4.18	2.62	0.01
25.00	4.29	2.72	0.00
30.00	4.29	2.72	0.00
35.00	4.29	2.72	0.00
40.00	4.29	2.72	0.00
45.00	4.29	2.72	0.00
50.00	4.29	2.72	0.00
55.00	4.29	2.72	0.00
60.00	4.29	2.72	0.00
65.00	4.29	2.72	0.00
70.00	4.29	2.72	0.00
75.00	4.29	2.72	0.00
80.00	4.29	2.72	0.00
85.00	4.29	2.72	0.00
90.00	4.29	2.72	0.00
95.00	4.29	2.72	0.00
100.00	4.29	2.72	0.00
105.00	4.29	2.72	0.00
110.00	4.29	2.72	0.00
115.00	4.29	2.72	0.00
120.00	4.29	2.72	0.00
125.00	4.29	2.72	0.00
130.00	4.29	2.72	0.00
135.00	4.29	2.72	0.00
140.00	4.29	2.72	0.00
145.00	4.29	2.72	0.00
150.00	4.29	2.72	0.00
155.00	4.29	2.72	0.00
160.00	4.29	2.72	0.00
165.00	4.29	2.72	0.00
170.00	4.29	2.72	0.00
175.00	4.29	2.72	0.00
180.00	4.29	2.72	0.00
185.00	4.29	2.72	0.00
190.00	4.29	2.72	0.00
195.00	4.29	2.72	0.00
200.00	4.29	2.72	0.00
205.00	4.29	2.72	0.00
210.00	4.29	2.72	0.00
215.00	4.29	2.72	0.00
220.00	4.29	2.72	0.00
225.00	4.29	2.72	0.00
230.00	4.29	2.72	0.00
235.00	4.29	2.72	0.00
240.00	4.29	2.72	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 60

Summary for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Runoff = 1.41 cfs @ 12.13 hrs, Volume= 0.067 af, Depth= 2.66"

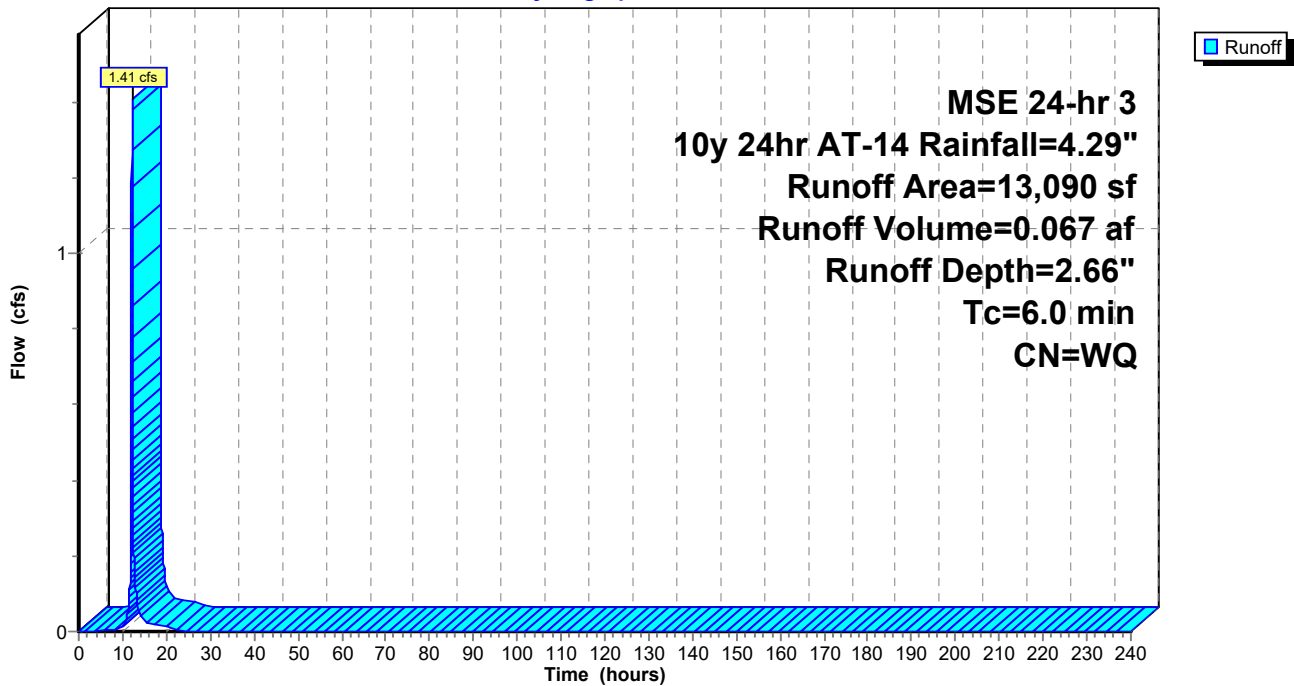
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Area (sf)	CN	Description
2,797	98	Paved parking, HSG D
10,293	80	>75% Grass cover, Good, HSG D
13,090		Weighted Average
10,293		78.63% Pervious Area
2,797		21.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 61

Hydrograph for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.16	0.00	0.00
10.00	0.59	0.02	0.01
15.00	3.85	2.24	0.04
20.00	4.18	2.53	0.01
25.00	4.29	2.63	0.00
30.00	4.29	2.63	0.00
35.00	4.29	2.63	0.00
40.00	4.29	2.63	0.00
45.00	4.29	2.63	0.00
50.00	4.29	2.63	0.00
55.00	4.29	2.63	0.00
60.00	4.29	2.63	0.00
65.00	4.29	2.63	0.00
70.00	4.29	2.63	0.00
75.00	4.29	2.63	0.00
80.00	4.29	2.63	0.00
85.00	4.29	2.63	0.00
90.00	4.29	2.63	0.00
95.00	4.29	2.63	0.00
100.00	4.29	2.63	0.00
105.00	4.29	2.63	0.00
110.00	4.29	2.63	0.00
115.00	4.29	2.63	0.00
120.00	4.29	2.63	0.00
125.00	4.29	2.63	0.00
130.00	4.29	2.63	0.00
135.00	4.29	2.63	0.00
140.00	4.29	2.63	0.00
145.00	4.29	2.63	0.00
150.00	4.29	2.63	0.00
155.00	4.29	2.63	0.00
160.00	4.29	2.63	0.00
165.00	4.29	2.63	0.00
170.00	4.29	2.63	0.00
175.00	4.29	2.63	0.00
180.00	4.29	2.63	0.00
185.00	4.29	2.63	0.00
190.00	4.29	2.63	0.00
195.00	4.29	2.63	0.00
200.00	4.29	2.63	0.00
205.00	4.29	2.63	0.00
210.00	4.29	2.63	0.00
215.00	4.29	2.63	0.00
220.00	4.29	2.63	0.00
225.00	4.29	2.63	0.00
230.00	4.29	2.63	0.00
235.00	4.29	2.63	0.00
240.00	4.29	2.63	0.00

Summary for Pond 1P: UNDERGROUND POND 1

Inflow Area = 1.687 ac, 83.48% Impervious, Inflow Depth = 3.76" for 10y 24hr AT-14 event
 Inflow = 10.16 cfs @ 12.13 hrs, Volume= 0.529 af
 Outflow = 6.18 cfs @ 12.20 hrs, Volume= 0.529 af, Atten= 39%, Lag= 4.1 min
 Primary = 6.18 cfs @ 12.20 hrs, Volume= 0.529 af

Routing by Stor-Ind method, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 Starting Elev= 965.40' Surf.Area= 4,484 sf Storage= 7,467 cf
 Peak Elev= 967.67' @ 12.20 hrs Surf.Area= 4,484 sf Storage= 14,720 cf (7,252 cf above start)

Plug-Flow detention time= 214.1 min calculated for 0.357 af (68% of inflow)
 Center-of-Mass det. time= 71.2 min (824.2 - 753.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	962.40'	0 cf	19.00'W x 236.00'L x 7.50'H Field A 33,630 cf Overall - 18,242 cf Embedded = 15,388 cf x 0.0% Voids
#2A	962.40'	18,242 cf	CMP Round 84 x 22 Inside #1 Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf Overall Size= 84.0"W x 84.0"H x 20.00'L 22 Chambers in 2 Rows 17.00' Header x 38.48 sf x 2 = 1,308.5 cf Inside
		18,242 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	961.90'	15.0" Round Culvert L= 103.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 961.90' / 961.38' S= 0.0050 '/' Cc= 0.900 n= 0.012 Concrete pipe, finished, Flow Area= 1.23 sf
#2	Device 1	965.40'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	966.40'	16.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=6.18 cfs @ 12.20 hrs HW=967.66' (Free Discharge)

- ↑ **1=Culvert** (Passes 6.18 cfs of 11.42 cfs potential flow)
- ↑ **2=Orifice/Grate** (Orifice Controls 0.94 cfs @ 6.90 fps)
- ↑ **3=Orifice/Grate** (Orifice Controls 5.24 cfs @ 3.83 fps)

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 63

Pond 1P: UNDERGROUND POND 1 - Chamber Wizard Field A

Chamber Model = CMP Round 84 (Round Corrugated Metal Pipe)

Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf

Overall Size= 84.0"W x 84.0"H x 20.00'L

84.0" Wide + 36.0" Spacing = 120.0" C-C Row Spacing

11 Chambers/Row x 20.00' Long +7.00' Header x 2 = 234.00' Row Length +12.0" End Stone x 2 = 236.00' Base Length

2 Rows x 84.0" Wide + 36.0" Spacing x 1 + 12.0" Side Stone x 2 = 19.00' Base Width

84.0" Chamber Height + 6.0" Stone Cover = 7.50' Field Height

22 Chambers x 769.7 cf + 17.00' Header x 38.48 sf x 2 = 18,241.7 cf Chamber Storage

33,630.0 cf Field - 18,241.7 cf Chambers = 15,388.3 cf Stone x 0.0% Voids = 0.0 cf Stone Storage

Chamber Storage = 18,241.7 cf = 0.419 af

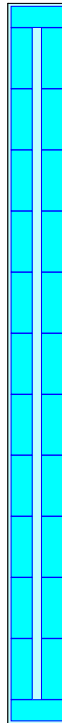
Overall Storage Efficiency = 54.2%

Overall System Size = 236.00' x 19.00' x 7.50'

22 Chambers

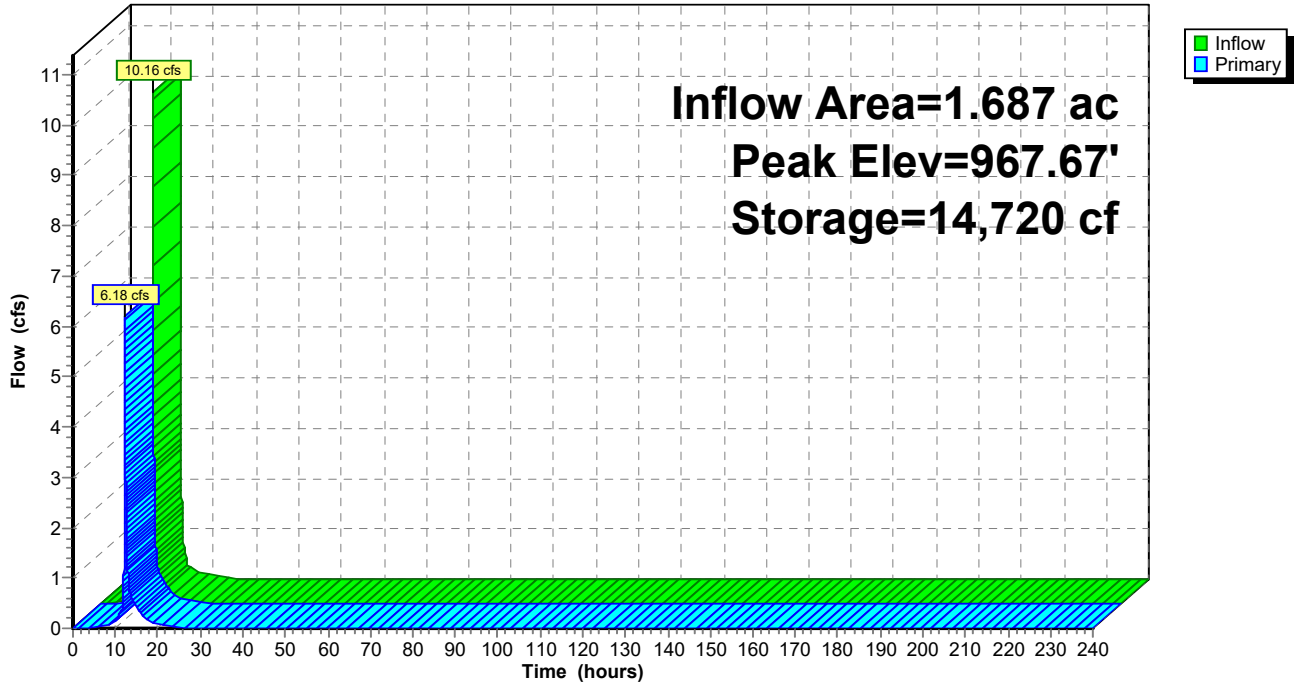
1,245.6 cy Field

569.9 cy Stone



Pond 1P: UNDERGROUND POND 1

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 65

Hydrograph for Pond 1P: UNDERGROUND POND 1

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
0.00	0.00	7,467	965.40	0.00
5.00	0.04	7,650	965.46	0.01
10.00	0.21	8,309	965.66	0.15
15.00	0.23	9,660	966.06	0.44
20.00	0.08	8,099	965.59	0.09
25.00	0.00	7,753	965.49	0.02
30.00	0.00	7,566	965.43	0.01
35.00	0.00	7,516	965.41	0.00
40.00	0.00	7,502	965.41	0.00
45.00	0.00	7,496	965.41	0.00
50.00	0.00	7,492	965.41	0.00
55.00	0.00	7,489	965.41	0.00
60.00	0.00	7,485	965.41	0.00
65.00	0.00	7,483	965.40	0.00
70.00	0.00	7,481	965.40	0.00
75.00	0.00	7,479	965.40	0.00
80.00	0.00	7,477	965.40	0.00
85.00	0.00	7,476	965.40	0.00
90.00	0.00	7,474	965.40	0.00
95.00	0.00	7,473	965.40	0.00
100.00	0.00	7,472	965.40	0.00
105.00	0.00	7,472	965.40	0.00
110.00	0.00	7,471	965.40	0.00
115.00	0.00	7,471	965.40	0.00
120.00	0.00	7,470	965.40	0.00
125.00	0.00	7,470	965.40	0.00
130.00	0.00	7,469	965.40	0.00
135.00	0.00	7,469	965.40	0.00
140.00	0.00	7,469	965.40	0.00
145.00	0.00	7,469	965.40	0.00
150.00	0.00	7,469	965.40	0.00
155.00	0.00	7,468	965.40	0.00
160.00	0.00	7,468	965.40	0.00
165.00	0.00	7,468	965.40	0.00
170.00	0.00	7,468	965.40	0.00
175.00	0.00	7,468	965.40	0.00
180.00	0.00	7,468	965.40	0.00
185.00	0.00	7,468	965.40	0.00
190.00	0.00	7,468	965.40	0.00
195.00	0.00	7,468	965.40	0.00
200.00	0.00	7,468	965.40	0.00
205.00	0.00	7,468	965.40	0.00
210.00	0.00	7,468	965.40	0.00
215.00	0.00	7,468	965.40	0.00
220.00	0.00	7,468	965.40	0.00
225.00	0.00	7,468	965.40	0.00
230.00	0.00	7,468	965.40	0.00
235.00	0.00	7,468	965.40	0.00
240.00	0.00	7,468	965.40	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 10y 24hr AT-14 Rainfall=4.29"

Printed 6/5/2024

Page 66

Stage-Area-Storage for Pond 1P: UNDERGROUND POND 1

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
962.40	0	965.00	6,168	967.60	14,531
962.45	12	965.05	6,328	967.65	14,675
962.50	52	965.10	6,490	967.70	14,818
962.55	96	965.15	6,652	967.75	14,960
962.60	148	965.20	6,814	967.80	15,100
962.65	207	965.25	6,977	967.85	15,239
962.70	271	965.30	7,140	967.90	15,376
962.75	341	965.35	7,303	967.95	15,511
962.80	416	965.40	7,467	968.00	15,644
962.85	495	965.45	7,632	968.05	15,776
962.90	578	965.50	7,797	968.10	15,906
962.95	666	965.55	7,961	968.15	16,034
963.00	757	965.60	8,127	968.20	16,160
963.05	851	965.65	8,292	968.25	16,284
963.10	949	965.70	8,458	968.30	16,406
963.15	1,050	965.75	8,623	968.35	16,526
963.20	1,155	965.80	8,789	968.40	16,643
963.25	1,262	965.85	8,955	968.45	16,758
963.30	1,371	965.90	9,121	968.50	16,870
963.35	1,484	965.95	9,287	968.55	16,980
963.40	1,599	966.00	9,453	968.60	17,087
963.45	1,716	966.05	9,618	968.65	17,191
963.50	1,835	966.10	9,784	968.70	17,292
963.55	1,957	966.15	9,950	968.75	17,390
963.60	2,081	966.20	10,115	968.80	17,485
963.65	2,207	966.25	10,280	968.85	17,576
963.70	2,335	966.30	10,445	968.90	17,663
963.75	2,465	966.35	10,610	968.95	17,747
963.80	2,597	966.40	10,774	969.00	17,826
963.85	2,731	966.45	10,938	969.05	17,901
963.90	2,866	966.50	11,102	969.10	17,970
963.95	3,003	966.55	11,265	969.15	18,035
964.00	3,142	966.60	11,428	969.20	18,093
964.05	3,282	966.65	11,590	969.25	18,145
964.10	3,423	966.70	11,752	969.30	18,189
964.15	3,566	966.75	11,913	969.35	18,222
964.20	3,711	966.80	12,074	969.40	18,242
964.25	3,856	966.85	12,234	969.45	18,242
964.30	4,003	966.90	12,393	969.50	18,242
964.35	4,151	966.95	12,552	969.55	18,242
964.40	4,301	967.00	12,710	969.60	18,242
964.45	4,451	967.05	12,867	969.65	18,242
964.50	4,603	967.10	13,023	969.70	18,242
964.55	4,755	967.15	13,178	969.75	18,242
964.60	4,909	967.20	13,333	969.80	18,242
964.65	5,063	967.25	13,486	969.85	18,242
964.70	5,219	967.30	13,639	969.90	18,242
964.75	5,375	967.35	13,791		
964.80	5,532	967.40	13,941		
964.85	5,690	967.45	14,090		
964.90	5,849	967.50	14,238		
964.95	6,008	967.55	14,385		

24132 PROPOSED*MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"*

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 67

Time span=0.00-240.00 hrs, dt=0.01 hrs, 24001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

SubcatchmentPR1A: DRAINS TO POND 1 Runoff Area=73,474 sf 83.48% Impervious Runoff Depth=6.83"
Tc=6.0 min CN=WQ Runoff=18.15 cfs 0.961 af

SubcatchmentPR1B: DRAINS SOUTH TO Runoff Area=14,493 sf 28.85% Impervious Runoff Depth=5.69"
Tc=6.0 min CN=WQ Runoff=3.23 cfs 0.158 af

SubcatchmentPR2: DRAINS NORTH TO Runoff Area=13,090 sf 21.37% Impervious Runoff Depth=5.53"
Tc=6.0 min CN=WQ Runoff=2.87 cfs 0.139 af

Pond 1P: UNDERGROUNDPOND 1 Peak Elev=969.30' Storage=18,187 cf Inflow=18.15 cfs 0.961 af
Outflow=11.30 cfs 0.961 af

Total Runoff Area = 2.320 ac Runoff Volume = 1.257 af Average Runoff Depth = 6.50"
32.40% Pervious = 0.752 ac 67.60% Impervious = 1.568 ac

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 68

Summary for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Runoff = 18.15 cfs @ 12.13 hrs, Volume= 0.961 af, Depth= 6.83"
 Routed to Pond 1P : UNDERGROUND POND 1

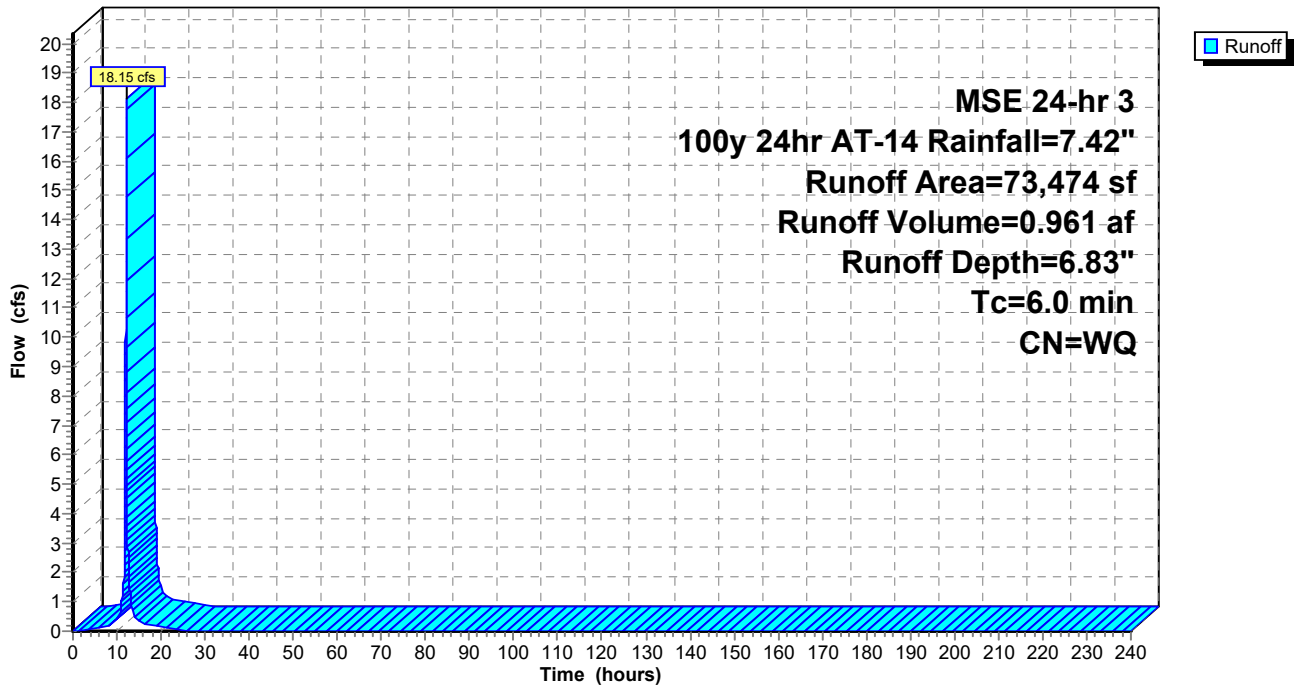
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Area (sf)	CN	Description
61,334	98	Paved parking, HSG D
12,140	80	>75% Grass cover, Good, HSG D
73,474		Weighted Average
12,140		16.52% Pervious Area
61,334		83.48% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 69

Hydrograph for Subcatchment PR1A: DRAINS TO POND 1 THEN SOUTH TO EX. STORM SEWER

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.28	0.04	0.10
10.00	1.02	0.58	0.39
15.00	6.66	6.06	0.40
20.00	7.23	6.63	0.13
25.00	7.42	6.82	0.00
30.00	7.42	6.82	0.00
35.00	7.42	6.82	0.00
40.00	7.42	6.82	0.00
45.00	7.42	6.82	0.00
50.00	7.42	6.82	0.00
55.00	7.42	6.82	0.00
60.00	7.42	6.82	0.00
65.00	7.42	6.82	0.00
70.00	7.42	6.82	0.00
75.00	7.42	6.82	0.00
80.00	7.42	6.82	0.00
85.00	7.42	6.82	0.00
90.00	7.42	6.82	0.00
95.00	7.42	6.82	0.00
100.00	7.42	6.82	0.00
105.00	7.42	6.82	0.00
110.00	7.42	6.82	0.00
115.00	7.42	6.82	0.00
120.00	7.42	6.82	0.00
125.00	7.42	6.82	0.00
130.00	7.42	6.82	0.00
135.00	7.42	6.82	0.00
140.00	7.42	6.82	0.00
145.00	7.42	6.82	0.00
150.00	7.42	6.82	0.00
155.00	7.42	6.82	0.00
160.00	7.42	6.82	0.00
165.00	7.42	6.82	0.00
170.00	7.42	6.82	0.00
175.00	7.42	6.82	0.00
180.00	7.42	6.82	0.00
185.00	7.42	6.82	0.00
190.00	7.42	6.82	0.00
195.00	7.42	6.82	0.00
200.00	7.42	6.82	0.00
205.00	7.42	6.82	0.00
210.00	7.42	6.82	0.00
215.00	7.42	6.82	0.00
220.00	7.42	6.82	0.00
225.00	7.42	6.82	0.00
230.00	7.42	6.82	0.00
235.00	7.42	6.82	0.00
240.00	7.42	6.82	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 70

Summary for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Runoff = 3.23 cfs @ 12.13 hrs, Volume= 0.158 af, Depth= 5.69"

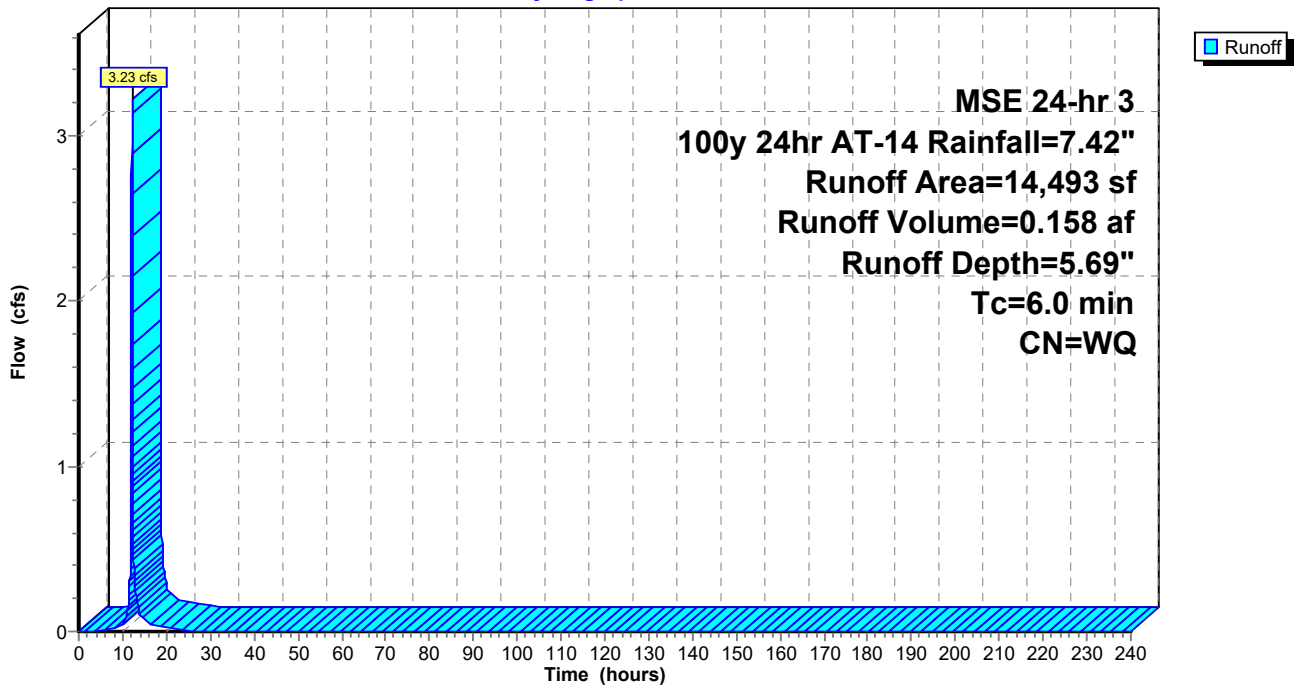
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Area (sf)	CN	Description
4,181	98	Paved parking, HSG D
10,312	80	>75% Grass cover, Good, HSG D
14,493		Weighted Average
10,312		71.15% Pervious Area
4,181		28.85% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 71

Hydrograph for Subcatchment PR1B: DRAINS SOUTH TO EX. STORM SEWER - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.28	0.00	0.01
10.00	1.02	0.18	0.04
15.00	6.66	4.92	0.08
20.00	7.23	5.47	0.03
25.00	7.42	5.65	0.00
30.00	7.42	5.65	0.00
35.00	7.42	5.65	0.00
40.00	7.42	5.65	0.00
45.00	7.42	5.65	0.00
50.00	7.42	5.65	0.00
55.00	7.42	5.65	0.00
60.00	7.42	5.65	0.00
65.00	7.42	5.65	0.00
70.00	7.42	5.65	0.00
75.00	7.42	5.65	0.00
80.00	7.42	5.65	0.00
85.00	7.42	5.65	0.00
90.00	7.42	5.65	0.00
95.00	7.42	5.65	0.00
100.00	7.42	5.65	0.00
105.00	7.42	5.65	0.00
110.00	7.42	5.65	0.00
115.00	7.42	5.65	0.00
120.00	7.42	5.65	0.00
125.00	7.42	5.65	0.00
130.00	7.42	5.65	0.00
135.00	7.42	5.65	0.00
140.00	7.42	5.65	0.00
145.00	7.42	5.65	0.00
150.00	7.42	5.65	0.00
155.00	7.42	5.65	0.00
160.00	7.42	5.65	0.00
165.00	7.42	5.65	0.00
170.00	7.42	5.65	0.00
175.00	7.42	5.65	0.00
180.00	7.42	5.65	0.00
185.00	7.42	5.65	0.00
190.00	7.42	5.65	0.00
195.00	7.42	5.65	0.00
200.00	7.42	5.65	0.00
205.00	7.42	5.65	0.00
210.00	7.42	5.65	0.00
215.00	7.42	5.65	0.00
220.00	7.42	5.65	0.00
225.00	7.42	5.65	0.00
230.00	7.42	5.65	0.00
235.00	7.42	5.65	0.00
240.00	7.42	5.65	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 72

Summary for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Runoff = 2.87 cfs @ 12.13 hrs, Volume= 0.139 af, Depth= 5.53"

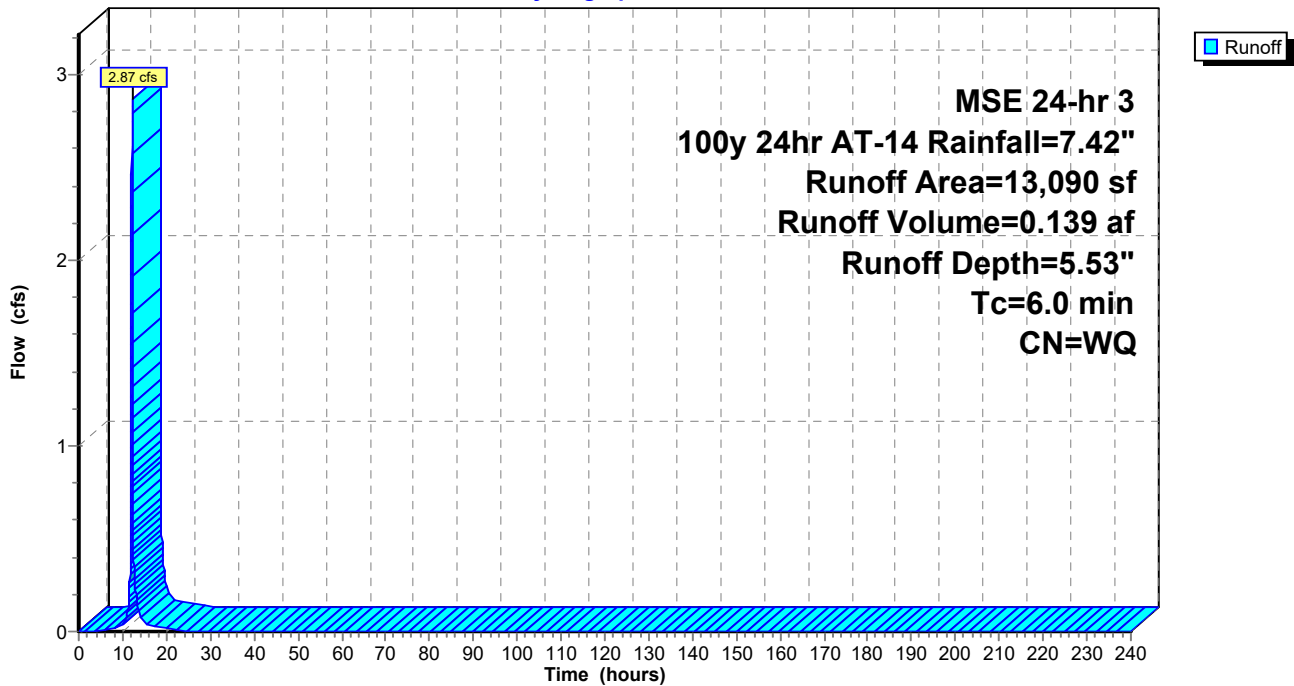
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Area (sf)	CN	Description
2,797	98	Paved parking, HSG D
10,293	80	>75% Grass cover, Good, HSG D
13,090		Weighted Average
10,293		78.63% Pervious Area
2,797		21.37% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 022202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 73

Hydrograph for Subcatchment PR2: DRAINS NORTH TO AMIRA APTS SITE - UNTREATED

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
0.00	0.00	0.00	0.00
5.00	0.28	0.00	0.00
10.00	1.02	0.16	0.04
15.00	6.66	4.81	0.07
20.00	7.23	5.36	0.02
25.00	7.42	5.54	0.00
30.00	7.42	5.54	0.00
35.00	7.42	5.54	0.00
40.00	7.42	5.54	0.00
45.00	7.42	5.54	0.00
50.00	7.42	5.54	0.00
55.00	7.42	5.54	0.00
60.00	7.42	5.54	0.00
65.00	7.42	5.54	0.00
70.00	7.42	5.54	0.00
75.00	7.42	5.54	0.00
80.00	7.42	5.54	0.00
85.00	7.42	5.54	0.00
90.00	7.42	5.54	0.00
95.00	7.42	5.54	0.00
100.00	7.42	5.54	0.00
105.00	7.42	5.54	0.00
110.00	7.42	5.54	0.00
115.00	7.42	5.54	0.00
120.00	7.42	5.54	0.00
125.00	7.42	5.54	0.00
130.00	7.42	5.54	0.00
135.00	7.42	5.54	0.00
140.00	7.42	5.54	0.00
145.00	7.42	5.54	0.00
150.00	7.42	5.54	0.00
155.00	7.42	5.54	0.00
160.00	7.42	5.54	0.00
165.00	7.42	5.54	0.00
170.00	7.42	5.54	0.00
175.00	7.42	5.54	0.00
180.00	7.42	5.54	0.00
185.00	7.42	5.54	0.00
190.00	7.42	5.54	0.00
195.00	7.42	5.54	0.00
200.00	7.42	5.54	0.00
205.00	7.42	5.54	0.00
210.00	7.42	5.54	0.00
215.00	7.42	5.54	0.00
220.00	7.42	5.54	0.00
225.00	7.42	5.54	0.00
230.00	7.42	5.54	0.00
235.00	7.42	5.54	0.00
240.00	7.42	5.54	0.00

24132 PROPOSED

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Prepared by Civil Site Group

Printed 6/5/2024

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

Page 74

Summary for Pond 1P: UNDERGROUND POND 1

Inflow Area = 1.687 ac, 83.48% Impervious, Inflow Depth = 6.83" for 100y 24hr AT-14 event
 Inflow = 18.15 cfs @ 12.13 hrs, Volume= 0.961 af
 Outflow = 11.30 cfs @ 12.20 hrs, Volume= 0.961 af, Atten= 38%, Lag= 4.0 min
 Primary = 11.30 cfs @ 12.20 hrs, Volume= 0.961 af

Routing by Stor-Ind method, Time Span= 0.00-240.00 hrs, dt= 0.01 hrs
 Starting Elev= 965.40' Surf.Area= 4,484 sf Storage= 7,467 cf
 Peak Elev= 969.30' @ 12.20 hrs Surf.Area= 4,484 sf Storage= 18,187 cf (10,719 cf above start)

Plug-Flow detention time= 154.6 min calculated for 0.789 af (82% of inflow)
 Center-of-Mass det. time= 54.7 min (801.0 - 746.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	962.40'	0 cf	19.00'W x 236.00'L x 7.50'H Field A 33,630 cf Overall - 18,242 cf Embedded = 15,388 cf x 0.0% Voids
#2A	962.40'	18,242 cf	CMP Round 84 x 22 Inside #1 Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf Overall Size= 84.0"W x 84.0"H x 20.00'L 22 Chambers in 2 Rows 17.00' Header x 38.48 sf x 2 = 1,308.5 cf Inside
		18,242 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	961.90'	15.0" Round Culvert L= 103.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 961.90' / 961.38' S= 0.0050 '/' Cc= 0.900 n= 0.012 Concrete pipe, finished, Flow Area= 1.23 sf
#2	Device 1	965.40'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	966.40'	16.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=11.29 cfs @ 12.20 hrs HW=969.29' (Free Discharge)

- ↑ **1=Culvert** (Passes 11.29 cfs of 13.13 cfs potential flow)
- ↑ **2=Orifice/Grate** (Orifice Controls 1.26 cfs @ 9.24 fps)
- ↑ **3=Orifice/Grate** (Orifice Controls 10.03 cfs @ 7.18 fps)

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 75

Pond 1P: UNDERGROUND POND 1 - Chamber Wizard Field A

Chamber Model = CMP Round 84 (Round Corrugated Metal Pipe)

Effective Size= 84.0"W x 84.0"H => 38.48 sf x 20.00'L = 769.7 cf

Overall Size= 84.0"W x 84.0"H x 20.00'L

84.0" Wide + 36.0" Spacing = 120.0" C-C Row Spacing

11 Chambers/Row x 20.00' Long +7.00' Header x 2 = 234.00' Row Length +12.0" End Stone x 2 = 236.00' Base Length

2 Rows x 84.0" Wide + 36.0" Spacing x 1 + 12.0" Side Stone x 2 = 19.00' Base Width

84.0" Chamber Height + 6.0" Stone Cover = 7.50' Field Height

22 Chambers x 769.7 cf + 17.00' Header x 38.48 sf x 2 = 18,241.7 cf Chamber Storage

33,630.0 cf Field - 18,241.7 cf Chambers = 15,388.3 cf Stone x 0.0% Voids = 0.0 cf Stone Storage

Chamber Storage = 18,241.7 cf = 0.419 af

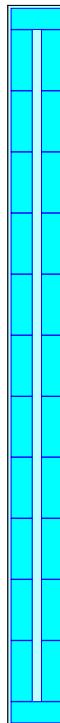
Overall Storage Efficiency = 54.2%

Overall System Size = 236.00' x 19.00' x 7.50'

22 Chambers

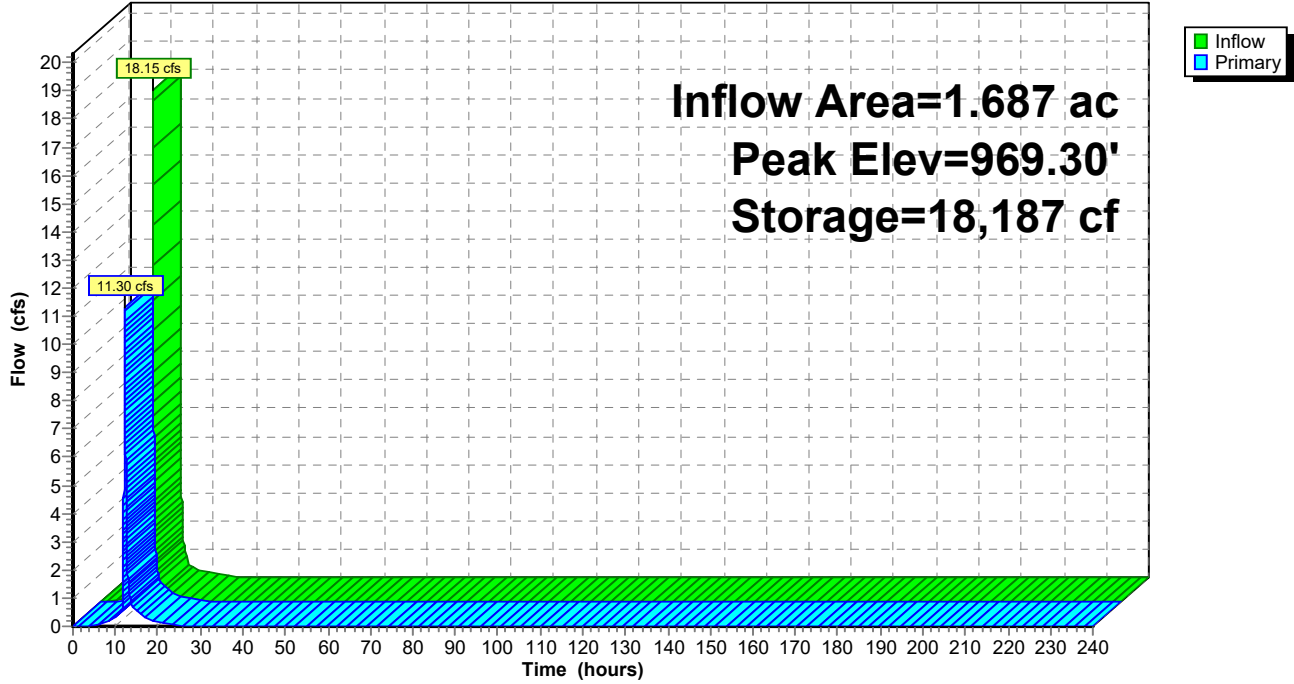
1,245.6 cy Field

569.9 cy Stone



Pond 1P: UNDERGROUND POND 1

Hydrograph



24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 77

Hydrograph for Pond 1P: UNDERGROUND POND 1

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
0.00	0.00	7,467	965.40	0.00
5.00	0.10	7,918	965.54	0.05
10.00	0.39	8,808	965.81	0.29
15.00	0.40	10,518	966.32	0.55
20.00	0.13	8,343	965.67	0.16
25.00	0.00	7,818	965.51	0.03
30.00	0.00	7,580	965.43	0.01
35.00	0.00	7,519	965.42	0.00
40.00	0.00	7,503	965.41	0.00
45.00	0.00	7,497	965.41	0.00
50.00	0.00	7,493	965.41	0.00
55.00	0.00	7,489	965.41	0.00
60.00	0.00	7,486	965.41	0.00
65.00	0.00	7,483	965.40	0.00
70.00	0.00	7,481	965.40	0.00
75.00	0.00	7,479	965.40	0.00
80.00	0.00	7,477	965.40	0.00
85.00	0.00	7,476	965.40	0.00
90.00	0.00	7,475	965.40	0.00
95.00	0.00	7,473	965.40	0.00
100.00	0.00	7,473	965.40	0.00
105.00	0.00	7,472	965.40	0.00
110.00	0.00	7,471	965.40	0.00
115.00	0.00	7,471	965.40	0.00
120.00	0.00	7,470	965.40	0.00
125.00	0.00	7,470	965.40	0.00
130.00	0.00	7,469	965.40	0.00
135.00	0.00	7,469	965.40	0.00
140.00	0.00	7,469	965.40	0.00
145.00	0.00	7,469	965.40	0.00
150.00	0.00	7,469	965.40	0.00
155.00	0.00	7,468	965.40	0.00
160.00	0.00	7,468	965.40	0.00
165.00	0.00	7,468	965.40	0.00
170.00	0.00	7,468	965.40	0.00
175.00	0.00	7,468	965.40	0.00
180.00	0.00	7,468	965.40	0.00
185.00	0.00	7,468	965.40	0.00
190.00	0.00	7,468	965.40	0.00
195.00	0.00	7,468	965.40	0.00
200.00	0.00	7,468	965.40	0.00
205.00	0.00	7,468	965.40	0.00
210.00	0.00	7,468	965.40	0.00
215.00	0.00	7,468	965.40	0.00
220.00	0.00	7,468	965.40	0.00
225.00	0.00	7,468	965.40	0.00
230.00	0.00	7,468	965.40	0.00
235.00	0.00	7,468	965.40	0.00
240.00	0.00	7,468	965.40	0.00

24132 PROPOSED

Prepared by Civil Site Group

HydroCAD® 10.20-4a s/n 02202 © 2023 HydroCAD Software Solutions LLC

MSE 24-hr 3 100y 24hr AT-14 Rainfall=7.42"

Printed 6/5/2024

Page 78

Stage-Area-Storage for Pond 1P: UNDERGROUND POND 1

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
962.40	0	965.00	6,168	967.60	14,531
962.45	12	965.05	6,328	967.65	14,675
962.50	52	965.10	6,490	967.70	14,818
962.55	96	965.15	6,652	967.75	14,960
962.60	148	965.20	6,814	967.80	15,100
962.65	207	965.25	6,977	967.85	15,239
962.70	271	965.30	7,140	967.90	15,376
962.75	341	965.35	7,303	967.95	15,511
962.80	416	965.40	7,467	968.00	15,644
962.85	495	965.45	7,632	968.05	15,776
962.90	578	965.50	7,797	968.10	15,906
962.95	666	965.55	7,961	968.15	16,034
963.00	757	965.60	8,127	968.20	16,160
963.05	851	965.65	8,292	968.25	16,284
963.10	949	965.70	8,458	968.30	16,406
963.15	1,050	965.75	8,623	968.35	16,526
963.20	1,155	965.80	8,789	968.40	16,643
963.25	1,262	965.85	8,955	968.45	16,758
963.30	1,371	965.90	9,121	968.50	16,870
963.35	1,484	965.95	9,287	968.55	16,980
963.40	1,599	966.00	9,453	968.60	17,087
963.45	1,716	966.05	9,618	968.65	17,191
963.50	1,835	966.10	9,784	968.70	17,292
963.55	1,957	966.15	9,950	968.75	17,390
963.60	2,081	966.20	10,115	968.80	17,485
963.65	2,207	966.25	10,280	968.85	17,576
963.70	2,335	966.30	10,445	968.90	17,663
963.75	2,465	966.35	10,610	968.95	17,747
963.80	2,597	966.40	10,774	969.00	17,826
963.85	2,731	966.45	10,938	969.05	17,901
963.90	2,866	966.50	11,102	969.10	17,970
963.95	3,003	966.55	11,265	969.15	18,035
964.00	3,142	966.60	11,428	969.20	18,093
964.05	3,282	966.65	11,590	969.25	18,145
964.10	3,423	966.70	11,752	969.30	18,189
964.15	3,566	966.75	11,913	969.35	18,222
964.20	3,711	966.80	12,074	969.40	18,242
964.25	3,856	966.85	12,234	969.45	18,242
964.30	4,003	966.90	12,393	969.50	18,242
964.35	4,151	966.95	12,552	969.55	18,242
964.40	4,301	967.00	12,710	969.60	18,242
964.45	4,451	967.05	12,867	969.65	18,242
964.50	4,603	967.10	13,023	969.70	18,242
964.55	4,755	967.15	13,178	969.75	18,242
964.60	4,909	967.20	13,333	969.80	18,242
964.65	5,063	967.25	13,486	969.85	18,242
964.70	5,219	967.30	13,639	969.90	18,242
964.75	5,375	967.35	13,791		
964.80	5,532	967.40	13,941		
964.85	5,690	967.45	14,090		
964.90	5,849	967.50	14,238		
964.95	6,008	967.55	14,385		